## Town of Johnstown

# PLANNING \& ZONING COMMISSION REGULAR MEETING 

Wednesday, November 01, 2023 at 7:00 PM
MISSION STATEMENT: Enhancing the quality of life of our residents, businesses, and visitors through community focused leadership.

## AGENDA

## CALL TO ORDER

## ROLL CALL

## APPROVAL OF AGENDA

## APPROVAL OF MINUTES

1. 8-23-23 Meeting Minutes

PUBLIC COMMENTS - Regarding items and issues not included as a Public Hearing on this Agenda (limited to 3 minutes each)

## NEW BUSINESS

2. SUB23-0003 Settler's Crossing
3. SUB23-0008 Blue Sky Prairie
4. ZON23-0001 Massey Square
5. ZON23-0002 Vista Commons

## DEPARTMENTAL REPORT

## COMMISSIONER REPORTS AND COMMENTS

## ADJOURN

## AMERICANS WITH DISABILITIES ACT NOTICE

In accordance with the Americans with Disabilities Act and other applicable laws, persons who need accommodation in order to attend or participate in this meeting should contact Town Hall at 970-587-4664 no later than 48 hours before the meeting in order to request such assistance.

De conformidad con la Ley de Discapacitados Estadounidenses y otras leyes vigentes, los individuos que necesitan adaptaciones funcionales para asistir o participar en esta reunión deberán comunicarse con la Municipalidad marcando el 970 587-4664 a lo más tardar 48 horas antes de dicha reunión para solicitarla.

## The Community That Cares

# PLANNING \& ZONING COMMISSION REGULAR MEETING 

7:00 PM, Wednesday, August 23, 2023

## SUMMARY MINUTES

The meeting was called to order by Chair Grentz at 7:00 P.M. Roll call attendance was taken. Present were Commissioners Flores, Hatfield, Hayward, Jeanneret, Urban, and Chairman Grentz. Campell was absent and excused.

Agenda was approved 6/0.

Minutes of the regular meetings held on July 26, 2023. Flores abstained rest of the members approved.

No public comment.

The Public Hearing for Final Development Plan for Southridge Subdivision was opened. Tyler Smith, Planner II Planning \& Development, presented a presentation and overview of the proposed 72-acre subdivision in The Ridge PUD. Commissioners inquired about the size of the streets in the plat, which Meyer clarified. Commissioners also questioned the detention pond which Smith clarified that the detention ponds. Smith noted that this would be a question for the engineers. Commissioners also questioned overall site accessibility. Meyer clarified these questions.

Morgan Kidder represented the Applicant and was available to answer any questions. Commission had questions involving pricing, long term relationships with Johnstown, and the material of the fields in the dentation pond. 6683 E CR 16, resident brought up concerns in relation to the fencing that South Ridge is to offer for the safety of their cattle. Commission asked questions of the fencing plan which was to be supplied with the Final Development Plan. Micheal asked about the home buyer programs that were going to be offered. Kidder clarified that he was not a part of the sales team but ensured that they had plans.

M: Hatfield / $2^{\text {nd: }}$ Flores
Vote: 6/0 to Recommend Approval with conditions

The Public Hearing for Preliminary Plat \& Development Plan for Revere North was opened. Tyler Smith, Planner II, Planning \& Development, presented a presentation and synopsis of the proposed 309.4-acre subdivision for townhomes in the Great Plains Village ODP located North of Veteran's Parkway and East of I-25. Commissioners inquired about parking requirements and garage styles present on elevations, along with detention and open spaces. Meyer clarified these questions.

James Hayes, VP with Forester, the Applicant was available to answer questions. The Commissioners had questions for the Applicant as for the retail opportunities that may occur in this development along with the size of the plat and traffic concerns. Kevin Lovelace, the applicants engineer answered

## The Community That Cares

questions regarding the drainage plan. Meyer responded to the traffic concern due to neighboring developments. Wendy Chase, 8445 E CR 13 concerns with weeds fire hazard, traffic hazards, along with the concern for trespassers and protection of her own property.

Mario Herrera, concerns about weeds and the fire hazard that this brought on, the traffic that is not able to meet the volume that Johnstown supplies, along with concerns with irrigation around his property for his use. Herrera also brought concerns with buffers and privacy that were brought along with the Development. The applicant answered questions involving weeds and shared irrigation use with Carreras's property.

Sandra Stoner, 4998 County Road location of Grassland Rd from Hyles driveway and property, traffic volume concerns, and overall speeding concerns with the presence of where these homes are located. Along with the concerns with the property values.

Jonah Heil, 4998 CR 50, concerns about the drainage points, with the impact of the properties that were there before the development due to flooding issues that have occurred.

Debroa Garcia, 4786 CR 50, concerns with the ROW extension to widen sections and who is responsible to supplying the land and the trees on the property. Meyer clarified these points.

Kevin Lovelace along with Jeff Mark, who represented the applicant supplied information on the Traffic concerns and processes to address concerns from the Commission and public.

Discussion occurred among Commission about the development before vote.

## M: Grentz / $2^{\text {nd: }}$ : Flores

Vote: Planning and Zoning Commission recommends approval 4/2

## Kim Meyer

Town Staff in attendance: Kim Meyer, Lilly Cory, Tyler Smith, Tony LeFevere
Kim Meyer noted the plans that were referenced during the meeting to further inform council.
Commissioners had no reports but asked an array of procedural, conduct, and general process questions. Meyer indicated there is some training intended for the commission and Staff will work on making that available to the full commission. Meeting was adjourned.

Respectfully Submitted
Accepted by Chair:
Lilly Cory, Planner I


# Planning \& Zoning Commission Staff Analysis Report 

ITEM:

## PROJECT:

PARCEL NO:
DESCRPTION:

LOCATION:
DEVELOPER:
STAFF:

HEARING DATE:

Public Hearing and Consideration of Settler's Crossing Preliminary/Final Subdivision Plat

SUB23-0003
106102100021
Commercial Subdivision of approximately 13.7 acres, in Mountain View West PD (2017)

South of Settler's Way \& East of Parish Ave
Parish, LLC
Lilly Cory, Planner I

## ATTACHMENTS:

1. Vicinity Map
2. Final Plat
3. Traffic Impact Study
4. Mountain View West PUD (2017)

## EXECUTIVE SUMMARY:

The Developer, Parish, LLC, with Land One Engineering is requesting approval of the Final Subdivision Plat within the Mountain View West PD. The subdivision will consist of 11 lots, dedicated right-of-way (ROW), and commercial signage. It is anticipated that this will act as neighborhood commercial with access to the surrounding residential community.
ZONING: PD- Mountain View West PUD (2017)

## ADJACENT ZONING \& LAND USE:

North: Mountain View West PUD - Residential
East: Mountain View West PUD - Residential
South: Johnstown Farm PD - Residential
West: Johnstown Town Hall \& Johnstown Farms - Residential

## PROPERTY HISTORY

The subject property was annexed in 2013 as a part of the Parish LLC annexation (Ordinance 2014-133). Zoning was done concurrently and (PUD B) was monumented by ordinance 2014134 and updated in 2017. Most of this PD has already been developed for residential use as Mountain View West Townhomes.

## SUBMITTAL AND REFERRALS

This project submittal included the proposed final subdivision plat, as well as required engineering plans and reports for the site. The project was referred to and reviewed by:

```
+ Weld County
+ IMEG (Town Engineer)
+ Little Thompson Water District
```

+ Front Range Fire District $\quad+$ FHU (Town Traffic Engineer)


## PROJECT DESCRIPITON \& ANALYSIS

The proposed subdivision would create 11 total lots for commercial businesses. The project is located East of Parish Ave, North of Centennial Dr or WCR 46.5, South of Settler's Way with one main access to the lots through Meadowlark Dr.

ROW will be landscaped and maintained by Parish LLC, as the WSSA will monument the total amount of water required. Other landscaping is to be maintained by the tenants as lots are developed.

At this time there are no public improvements that have been proposed or otherwise stated in the Development Agreement at this time.

The staff has no outstanding concerns with this subdivision and believes that it will promote the Town's housing diversity and local economy. There are no concerns of incompatibility with surrounding Town Developments \& Zoning.

## PUBLIC NOTICE

Notice for the Planning and Zoning Meeting and Public Hearing was published in the Johnstown Breeze on Thursday October 5, 2023. This notice provided the date, time, and location of the hearing along with a description of the project. Notices were mailed out on Thursday September 28,2023 , to all landowners in an 800 ft radius from the property. Notification was sent out once again due to previously cancelled meeting for both the Johnstown Breeze and property owners on October 19 ${ }^{\text {th }}, 2023$.

## The Community That Cares

johnstown.colorado.gov

## RECOMMENDED FINDINGS AND MOTIONS

## Recommended Findings

It is recommended that the Planning and Zoning Commission send a recommendation for approval to Town Council for the Preliminary/Final Subdivision Plat based upon the following findings:
1.) The proposed subdivision is consistent with the town comprehensive plan and is in compliance with the Town's codes, regulations, and requirements along with the Mountain View West PD.

## Recommended Motion to Approve

I move to approve Settler's Crossing Preliminary/Final Subdivision Plat, based on the application received, information provided, and findings noted at this hearing.

## Alternative Motions:

## Motion to Recommend Approval with Conditions

I move to approve Settler's Crossing Preliminary/Final Subdivision Plat with conditions, based on the application received, information provided, and findings noted at this hearing. The conditions are listed as such:

## Motion to Recommend Denial

I move that the Commission recommend to Town Council Denial of the Settler's Crossing Preliminary/Final Subdivision Plat, based on the following findings.


## SETTLER'S CROSSING SUBDIVISION

Being a Replat of Block 3 of Mountain View West Subdivision Replat Amendment No. 1
Town of Johnstown, County of Weld, State of Colorado
cesaricare of ofocation




omeres appound


Conid s.e.
 State of Collio
county
$\qquad$ _) ${ }^{38}$



Pboce Notor soor Abone
$\qquad$
${ }^{\text {rown councll }}$



By Nower



TIEE Commenerv Nore

surweors noiss

 The ineold dimensions sos osontioned hereine ore bosese uoon the us. Sureer foot

puat iots














## suncoors smaterit



1 $\qquad$

PRELIMINARY

LAT40, Inc.
LAT $40^{\circ}$, Inc.
Professional Land Surveyors 6250 W. 10th Street, Unit 2 Greeley, CO 80634
O: $970-515-5294$ O: 970-515-5294


SETTLER'S CROSSING SUBDIVISION
$\square$ MOUNTAIN VIEW WEST SUBDIVISION - JOHNSTOWN
 WELD COUNTY - SECTION 9, TAN, R6TW.


## SETTLER'S CROSSING SUBDIVISION

Being a Replat of Block 3 of Mountain View West Subdivision Replat Amendment No. 1


| To: | David Gilbert, Parish, LLC |
| :--- | :--- |
| From: | Eli Farney, PE, PTOE |
| Date: | May 1, 2023 |

# Mountain View Commercial Development 

> Johnstown, Colorado

Prepared By:

## J•R ENGINEERING

Eli Farney, PE, PTOE
efarney@jrengineering.com
JR Engineering
7200 South Alton Way, Suite C400
Centennial, CO 80112

## Table of Contents

Executive Summary ..... 3
Introduction ..... 4
Traffic Volumes and Distribution ..... 7
Traffic Operations Analysis ..... 15
Conclusion ..... 24
Appendix ..... 25
List of Figures
Figure 1: Vicinity Map ..... 4
Figure 2: Study Intersections and Site Plan ..... 5
Figure 3: Site-Generated Traffic Distribution ..... 8
Figure 4: Existing (2022) Traffic Volumes ..... 9
Figure 5: Site-Generated Traffic Volumes ..... 10
Figure 6: Opening Day (2024) Background Traffic Volumes ..... 11
Figure 7: Opening Day (2024) Total Traffic Volumes ..... 12
Figure 8: Future Year (2045) Background Traffic Volumes ..... 13
Figure 9: Future Year (2045) Total Traffic Volumes ..... 14
Figure 10: Peak Hour Traffic Signal Warrant ..... 22
List of Tables
Table 1: Traffic Modeling Parameters ..... 15
Table 2: 2022 (Existing) Levels of Service ..... 16
Table 3: 2024 (Opening Day) Levels of Service ..... 17
Table 4: 2045 (Future Year) Levels of Service ..... 18
Table 5: 2022 (Existing) 95 ${ }^{\text {th }}$ Percentile Queue Lengths ..... 19
Table 6: 2024 (Opening Day) 95 ${ }^{\text {th }}$ Percentile Queue Lengths ..... 20
Table 7: 2045 (Future Year) 95 ${ }^{\text {th }}$ Percentile Queue Lengths ..... 21
List of Appendices
Appendix A: Traffic Counts ..... 25
Appendix B: Trip Generation ..... 34
Appendix C: Synchro Reports ..... 37

## Executive Summary

JR Engineering (JR) has completed a review of the traffic impacts resulting from the proposed Mountain View Commercial Development (Project) in Johnstown, Colorado (Town).

The objectives of this Traffic Impact Study (TIS, Study) are:

- Estimate site-generated traffic and route trips onto adjacent streets
- Analyze 2024 (Opening Day) and 2045 (Future Year) traffic operations
- Make recommendations for improvements to accommodate future traffic volumes

The methodology, content, and findings of this TIS are consistent with the following documents:

- Weld County Engineering and Construction Criteria - Chapter 8.1 - Traffic Impact Studies


## Key Findings of this TIS

- Levels of Service
- All movements operate at LOS C or better in 2022.
- Most movements are expected to operate at LOS D or better in 2024 with both background traffic and total traffic.
- In 2045, most movements are expected to operate at LOS D or better with total traffic. A few movements are expected to fail at the Parish Avenue intersections.
- Queue Lengths
- Queue lengths are mostly anticipated to be satisfactory in 2045. Queuing at Parish \& Settler for the NBT and SBT movement may block access to driveways. Queuing for the SBL movement at this intersection may exceed the existing storage length.
- Improvements
- The Parish \& Settler intersection may become signalized in the future.
- WCR 46.5 may be widened in the future.
- Additional turn lanes along WCR 46.5 may be needed to improve traffic operations.


## Introduction

JR has completed a review of the existing and forecasted traffic operations in the vicinity of the Mountain View Commercial Development. A vicinity map is included in Figure 1.


Figure 1: Vicinity Map

## Land Uses

The development is anticipated to contain the following land uses:

- Shopping Plaza (40-150k) - ITE 821
- 110,000 square feet
- No supermarket


## Study Intersections

JR analyzed four intersections external to the Project site. These intersections are listed below and shown in Figure 2.

Study intersections:

1. Parish Avenue \& Molinar Street
2. Parish Avenue \& Settler Way
3. Parish Avenue \& WCR 46.5/Centennial Drive
4. WCR 46.5 \& Mountain Bluebird Drive


Figure 2: Study Intersections and Site Plan

## Lane Geometry and Intersection Control

## Assumptions

For the purposes of this Study, JR assumed that existing lane geometry would remain for the future scenarios, with the following exceptions:

- The intersection of WCR 46.5 \& Mountain Bluebird Dr is currently a T-intersection, but was modeled with a south leg in the future scenarios.
- Additional turn lanes were modeled along WCR 46.5 in 2045.


## Future Widening of WCR 46.5

WCR 46.5 to the south of the Project site may be widened in the future. This would be a Town project. In this Study, it was assumed that additional turn lanes along WCR 46.5 would be provided by 2045.

## Signalization of Parish \& Settler

JR conducted a signal warrant analysis determining that signalization may be warranted at the Parish \& Settler intersection by 2045. For the purposes of this Study, it is assumed that this intersection will become signalized by 2045 .

## Traffic Volumes and Distribution

## Existing Traffic Volumes

Existing traffic volumes were obtained on Thursday, October 6, 2022 by All Traffic Data Services for each of the Study intersections. Existing traffic volumes are included in Figure 4. Traffic counts are included in Appendix A.

## Background Traffic

JR estimated background traffic volumes by applying a 3\% growth rate to the existing traffic volumes to account for future regional development. This growth rate is based on data from the NFRMPO regional travel demand model.

Background traffic also includes estimated site-generated traffic from the Mountain View Residential Development (analyzed by JR on October 28, 2022), as well as a planned 143-unit multi-family development to the north of Molinar Street.

Future background traffic volumes are shown in Figure 6 (2024) and Figure 8 (2045).

## Site-Generated Traffic Volumes

Site-generated traffic volumes were estimated using ITE Trip Generation Manual, $11^{\text {th }}$ Edition. The development is expected to produce the following trips:

- Average Daily Trips: 7,427
- AM Peak Entering Site: 118
- AM Peak Exiting Site: 72
- PM Peak Entering Site: 280
- PM Peak Exiting Site: 291

Site-generated traffic volumes are shown in Figure 5. A trip generation report is included in Appendix B.

## Distribution of Site-Generated Traffic

Site-generated traffic was routed onto adjacent streets according to the distribution in Figure 3. The distribution is based on existing traffic volumes.


Figure 3: Site-Generated Traffic Distribution

## Total Traffic

Total traffic is the sum of background and site-generated traffic. JR forecasted total traffic volumes at the Study intersections in the years 2024 (Opening Day) and 2045 (Future Year). Total traffic volumes are shown in Figure 7 (2024) and Figure 9 (2045).

Existing traffic volumes at the study intersections are included in Figure 4. Existing lane geometry is shown


Figure 4: Existing (2022) Traffic Volumes

Site-generated traffic volumes at the study intersections are included in Figure 5.


Figure 5: Site-Generated Traffic Volumes


Figure 6: Opening Day (2024) Background Traffic Volumes

2024 total traffic volumes at the study intersections are included in Figure 7. Lane geometry is shown.


Figure 7: Opening Day (2024) Total Traffic Volumes

2045 background traffic volumes at the study intersections are included in Figure 8. Lane geometry is shown.


Figure 8: Future Year (2045) Background Traffic Volumes

2045 total traffic volumes at the study intersections are included in Figure 9. Lane geometry is shown


Figure 9: Future Year (2045) Total Traffic Volumes

## Traffic Operations Analysis

Traffic operations were analyzed using HCM 6 ${ }^{\text {th }}$ Edition methodology. Synchro reports are included in Appendix C.

## Traffic Modeling Parameters

JR considered traffic modeling parameters such as peak hour factor and heavy vehicle percentage. Table 1 summarizes the parameters considered, and the justification for values used. The values for these parameters are contained within the Synchro reports in Appendix C.

Table 1: Traffic Modeling Parameters

| Parameter | Justification |
| :--- | :--- |
| Peak Hour Factor (existing) | For existing traffic volumes (2022), JR used peak hour factors counted <br> by All Traffic Data Services. |
| Peak Hour Factor (future) | For future traffic volumes (2024 and 2045), JR used values suggested by <br> the Synchro 11 software, which are based on a Poisson distribution. |
| Heavy Vehicle Percentage | JR assumed 2\% heavy vehicles at all Study intersections, which is <br> consistent with the values counted by All Traffic Data Services. |
| Saturated Flow Rate (protected) | JR used values calculated in the Synchro 11 software, which are based <br> on HCM 6 |
| Saturated Flition. |  |

## Levels of Service

JR analyzed each of the Study intersections for peak hour level of service (LOS). Table 2 includes the LOS for each movement in the existing condition (2022). Table 3 includes the forecasted LOS for background traffic and total traffic in the year 2024. Table 4 includes the forecasted LOS for background traffic and total traffic in the year 2045. In each table, seconds of delay are shown in parentheses for movements operating at LOS F.

Table 2: 2022 (Existing) Levels of Service

| STOP | Intersection | Movement/Approach | AM Peak LOS | $\begin{aligned} & \text { PM Peak } \\ & \text { LOS } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: |
|  | 1: Parish Avenue \& Molinar Street | WB Left | A | C |
|  |  | WB Right | B | B |
|  |  | SB Approach | A | A |
| STOP | 2: Parish Avenue \& Settler Way | EB Left | C | C |
|  |  | EB Through/Right | B | B |
|  |  | WB Left | C | C |
|  |  | WB Through/Right | B | B |
|  |  | NB Left | A | A |
|  |  | SB Left | A | A |
| STOP | 3: Parish Avenue \& WCR 46.5 / Centennial Drive | EB Approach | B | B |
|  |  | WB Approach | B | B |
|  |  | NB Left | A | A |
|  |  | SB Left | A | A |
| STOP | 4: WCR 46.5 \& Mountain Bluebird Drive | EB Approach | A | A |
|  |  | SB Left | A | A |
|  |  | SB Right | A | A |

Table 3: 2024 (Opening Day) Levels of Service


Table 4: 2045 (Future Year) Levels of Service

| STOP | Intersection | Movement/ <br> Approach | AM Peak LOS |  | PM Peak LOS |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Background Traffic | Total Traffic | Background Traffic | Total <br> Traffic |
|  | 1: Parish Avenue <br> \& Molinar Street | WB Left | D | D | F (61s) | F (136s) |
|  |  | WB Right | B | B | C | C |
|  |  | SB Approach | A | A | A | B |
|  | 2: Parish Avenue \& Settler Way | EB Left | B | C | C | C |
|  |  | EB Through/Right | B | C | C | C |
|  |  | WB Left | B | B | C | C |
|  |  | WB Through/Right | C | C | C | F (88s) |
|  |  | NB Left | B | B | B | B |
|  |  | NB Through | B | B | C | D |
|  |  | NB Right | B | B | B | C |
|  |  | SB Left | B | B | B | D |
|  |  | SB Through | B | B | B | B |
|  |  | SB Right | A | A | A | B |
|  |  | Overall | B | B | B | D |
| STOP | 3: Parish Avenue \& WCR 46.5 / Centennial Drive | EB Approach | D | E | F (70s) | F (105s) |
|  |  | WB Left | D | E | F (59s) | F (199s) |
|  |  | WB Through | C | C | E | E |
|  |  | WB Right | B | B | B | B |
|  |  | NB Left | A | A | A | A |
|  |  | SB Left | A | A | A | A |
| STOP | 4: WCR 46.5 \& Mountain Bluebird Drive | EB Left | A | A | A | A |
|  |  | WB Approach | A | A | A | A |
|  |  | NB Left | A | A | A | A |
|  |  | NB Through/Right | A | A | A | A |
|  |  | SB Left | B | B | B | B |
|  |  | SB Through/Right | A | A | A | A |

## Discussion on Levels of Service

In the existing condition, all movements operate at LOS C or better.

In the 2024 condition, most movements are expected to operate at LOS D or better with both background traffic and total traffic. At the Parish \& Settler intersection, the EBL and WBL movements are expected to operate at LOS F in the PM peak hour with total traffic.

In the 2045 condition, most movements are expected to operate at LOS D or better. At the Parish \& Settler intersection, some movements are likely to improve as a result of signalization.

A few failing movements are anticipated at the Study intersections in 2045. These failures occur on east/west approaches to the intersections along Parish Avenue. Particularly long delays are expected at WCR 46.5 \& Parish due to this being an arterial-arterial intersection with two-way stop control. This intersection could become signalized in the future to improve levels of service.

## Queue Lengths

JR analyzed each of the Study intersections for $95^{\text {th }}$ percentile queue lengths using HCM $6^{\text {th }}$ Edition methodology. Table 5 includes the queue lengths for the year 2022 with existing traffic. Table 6 includes the queue lengths for the year 2024 with total traffic. Table 7 includes the queue lengths for the year 2045 with total traffic.

Table 5: 2022 (Existing) 95 ${ }^{\text {th }}$ Percentile Queue Lengths

| Intersection | Movement/Approach | AM Peak Queue (ft) | PM Peak Queue (ft) |
| :---: | :---: | :---: | :---: |
| 1: Parish Avenue \& Molinar Street | WB Left | <25 | <25 |
|  | WB Right | <25 | <25 |
|  | SB Approach | <25 | <25 |
| 2: Parish Avenue \& Settler Way | EB Left | <25 | <25 |
|  | EB Through/Right | <25 | <25 |
|  | WB Left | <25 | <25 |
|  | WB Through/Right | <25 | <25 |
|  | NB Left | <25 | <25 |
|  | SB Left | <25 | <25 |
| 3: Parish Avenue \& WCR 46.5 / Centennial Drive | EB Approach | <25 | <25 |
|  | WB Approach | <25 | <25 |
|  | NB Left | <25 | <25 |
|  | SB Left | <25 | <25 |
| 4: WCR 46.5 \& Mountain Bluebird Drive | EB Approach | <25 | <25 |
|  | SB Left | <25 | <25 |
|  | SB Right | <25 | <25 |

Table 6: 2024 (Opening Day) 95 ${ }^{\text {th }}$ Percentile Queue Lengths

|  | Intersection | Movement/Approach | AM Peak Queue (ft) | PM Peak Queue (ft) |
| :---: | :---: | :---: | :---: | :---: |
|  | 1: Parish Avenue \& Molinar Street | WB Left | <25 | <25 |
|  |  | WB Right | <25 | <25 |
|  |  | SB Approach | <25 | <25 |
| STOP | 2: Parish Avenue \& Settler Way | EB Left | <25 | <25 |
|  |  | EB Through/Right | <25 | <25 |
|  |  | WB Left | <25 | 88 |
|  |  | WB Through/Right | <25 | 60 |
|  |  | NB Left | <25 | <25 |
|  |  | SB Left | <25 | 25 |
| STOP | 3: Parish Avenue \& WCR 46.5 / Centennial Drive | EB Approach | <25 | <25 |
|  |  | WB Approach | <25 | 40 |
|  |  | NB Left | <25 | <25 |
|  |  | SB Left | <25 | <25 |
| STOP | 4: WCR 46.5 \& Mountain Bluebird Drive | EB Approach | $<25$ | $<25$ |
|  |  | WB Approach | <25 | <25 |
|  |  | NB Left | $<25$ | $<25$ |
|  |  | NB Through/Right | <25 | $<25$ |
|  |  | SB Left | <25 | $<25$ |
|  |  | SB Through/Right | <25 | <25 |

Table 7: 2045 (Future Year) 95 ${ }^{\text {th }}$ Percentile Queue Lengths

|  | Intersection | Movement/Approach | AM Peak Queue (ft) | PM Peak Queue (ft) |
| :---: | :---: | :---: | :---: | :---: |
|  | 1: Parish Avenue \& Molinar Street | WB Left | <25 | 45 |
|  |  | WB Right | $<25$ | <25 |
|  |  | SB Approach | <25 | <25 |
|  | 2: Parish Avenue \& Settler Way | EB Left | <25 | 25 |
|  |  | EB Through/Right | <25 | <25 |
|  |  | WB Left | 32 | 65 |
|  |  | WB Through/Right | 37 | 41 |
|  |  | NB Left | <25 | <25 |
|  |  | NB Through | 294 | 458 |
|  |  | NB Right | <25 | <25 |
|  |  | SB Left | 70 | 252 |
|  |  | SB Through | 259 | 419 |
|  |  | SB Right | <25 | <25 |
|  | 3: Parish Avenue \& WCR 46.5 / Centennial Drive | EB Approach | 80 | 115 |
|  |  | WB Left | 40 | 150 |
|  |  | WB Through | <25 | <25 |
|  |  | WB Right | <25 | <25 |
|  |  | NB Left | <25 | <25 |
|  |  | SB Left | <25 | <25 |
| STOP | 4: WCR 46.5 \& Mountain Bluebird Drive | EB Left | <25 | <25 |
|  |  | WB Approach | <25 | <25 |
|  |  | NB Left | <25 | <25 |
|  |  | NB Through/Right | <25 | <25 |
|  |  | SB Left | <25 | <25 |
|  |  | SB Through/Right | <25 | <25 |

## Discussion on Queue Lengths

Queue lengths are expected to be nominal in 2024 with total traffic. In 2045, most queue lengths are expected to be satisfactory. At Parish \& Settler, queuing for the SBT movement may block access to the driveway serving the police department in both peak hours. Similarly, queuing for the NBT movement may block access to the driveway serving Town Hall in the PM peak hour.

Most queue lengths for turning movements are expected to fit within existing storage. However, queuing for the SBL movement at Parish \& Settler is expected to exceed the current storage length of about 170 feet in the PM peak hour. Modification to this turn lane may be needed.

## Traffic Signal Warrant Analysis

JR performed a preliminary traffic signal warrant analysis on the intersection of Parish \& Settler. Specifically, JR checked the peak hour signal warrant according to the MUTCD. Traffic volumes were plotted on Figure 4C-3 of the MUTCD, which is shown in Figure 10. JR considered all volumes on the westbound approach and used the " 2 or more lanes \& 2 or more lanes" curve on the graph.


Figure 10: Peak Hour Traffic Signal Warrant

With 2024 total traffic volumes, the intersection is not expected to meet the peak hour warrant for signalization. With 2045 total traffic volumes, the intersection is expected to meet the warrant. Therefore, the warrant is anticipated to be met sometime between 2024 and 2045.

Due to the limited applicability of the peak hour warrant, JR also considered the potential to meet the 4-hour warrant. JR estimated hourly traffic volumes by using ITE data for hourly distribution of vehicle trips. Based on a preliminary analysis of the 4-hour warrant, JR concludes that that the warrant is unlikely to be met by 2024. However, the 4-hour warrant is anticipated to be met sometime before 2045.

JR believes that Parish \& Settler is an ideal location for a signal, as it could improve safety for accessing Town Hall, police department, library, and YMCA. Ongoing warrant studies should be conducted to monitor traffic conditions at this intersection. It should become signalized once warrants are met. Future coordination with Town staff will be necessary to determine responsibilities for signal construction costs.

## Turn Lanes along WCR 46.5

JR gave consideration to the need for turn lanes along WCR 46.5 in the future. At the intersection of WCR 46.5 \& Parish, turning movement volumes are higher than through movement volumes along WCR 46.5. Therefore, additional turn lanes may improve traffic operations.

JR reviewed the CDOT State Highway Access Code to determine whether turn lanes may be necessary. For non-rural arterials, left turn lanes are required when the turning volume is greater than 25 vehicles in the peak hour. Right turn lanes are required when the turning volume is greater than 50 vehicles in the peak hour.

Based on these standards, the following movements would require auxiliary lanes by 2045:

- WCR 46.5 \& Parish Avenue
- Eastbound left
- Westbound left
- Westbound right
- WCR 46.5 \& Mountain Bluebird Drive
- Eastbound left
- Westbound right

Based on consideration of both traffic operations and CDOT Access Code requirements, JR recommends the following turn lanes be installed in the future:

- Westbound left at WCR 46.5 \& Parish
- Westbound right at WCR 46.5 \& Parish
- Eastbound left at WCR 46.5 \& Mountain Bluebird

These improvements would be jointly funded between the Town and the future development to the south, since the north half of WCR 46.5 is built.

## Conclusion

Below is a summary of the conclusions and findings of this TIS.

## Levels of Service

All movements operate at LOS C or better in 2022. Most movements are expected to operate at LOS D or better in 2024 with both background traffic and total traffic. In 2045, most movements are expected to operate at LOS D or better, with a few movements anticipated to fail at the Parish Avenue intersections.

## Queue Lengths

Queue lengths are mostly expected to be satisfactory in year 2045. The NBT and SBT movements at Parish \& Settler may limit access to driveways. Also, queuing for the SBL movement at this intersection may exceed the existing storage length.

## Improvements

A few proposed improvements may help traffic operations at the Study intersections. Specifically, JR assumed that the intersection of Parish \& Settler would become signalized by 2045. Also, WCR 46.5 may be widened in the future, possibly with additional turn lanes. Improvements to WCR 46.5 would be jointly funded between the Town and the future development to the south.

# Appendix A Traffic Counts 

Location: 1 PARISH AVE (WCR17) \& MOLINAR ST AM
Date: Thursday, October 6, 2022
Peak Hour: 07:15 AM - 08:15 AM
Peak 15-Minutes: 08:00 AM - 08:15 AM

## Peak Hour - Pedestrians/Bicycles on Crosswalk

Peak Hour - All Vehicles



Note: Total study counts contained in parentheses.
Traffic Counts


Location: 2 PARISH AVE (WCR17) \& SETTLER WAY AM
Date: Thursday, October 6, 2022
Peak Hour: 07:15 AM - 08:15 AM
Peak 15-Minutes: 08:00 AM - 08:15 AM


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval | SETTLER WAY Eastbound |  |  |  | SETTLER WAY <br> Westbound |  |  |  | PARISH AVE (WCR17) <br> Northbound |  |  |  | PARISH AVE (WCR17) <br> Southbound |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn |  | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 7:00 AM | 0 | 3 | 1 | 2 | 0 | 0 | 0 | 13 | 0 | 1 | 89 | 1 | 0 | 11 | 28 | 2 | 151 | 562 | 0 | 1 | 0 | 0 |
| 7:15 AM | 0 | 4 | 1 | 1 | 0 | 1 | 1 | 4 | 0 | 1 | 65 | 2 | 0 | 9 | 46 | 3 | 138 | 577 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 2 | 1 | 2 | 0 | 1 | 0 | 9 | 0 | 1 | 62 | 3 | 0 | 9 | 54 | 5 | 149 | 548 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 1 | 1 | 0 | 0 | 3 | 0 | 13 | 0 | 0 | 48 | 2 | 0 | 15 | 35 | 6 | 124 | 543 | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 1 | 0 | 1 | 0 | 2 | 0 | 22 | 0 | 1 | 40 | 1 | 0 | 16 | 75 | 7 | 166 | 516 | 1 | 0 | 0 | 0 |
| 8:15 AM | 0 | 2 | 1 | 1 | 0 | 6 | 1 | 7 | 0 | 1 | 45 | 1 | 0 | 4 | 37 | 3 | 109 |  | 0 | 2 | 1 | 0 |
| 8:30 AM | 0 | 1 | 0 | 0 | 0 | 3 | 0 | 10 | 0 | 0 | 79 | 6 | 0 | 10 | 33 | 2 | 144 |  | 2 | 0 | 0 | 0 |
| 8:45 AM | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 11 | 0 | 0 | 45 | 4 | 0 | 9 | 25 | 1 | 97 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 15 | 5 | 7 | 0 | 17 | 2 | 289 | 0 | 5 | 473 | 20 | 0 | 83 | 333 | 29 | 1,078 |  | 3 | 3 | 1 | 0 |
| Peak Hour | 0 | 8 | 3 | 4 | 0 | 7 | 1 | 48 | 0 | 3 | 215 | 8 | 0 | 49 | 210 | 21 | 577 | 77 | 1 | 0 | 0 | 0 |

Location: 3 PARISH AVE (WCR17) \& CENTENNIAL DR (WCR 46.5) AM
Date: Thursday, October 6, 2022
ALL TRAFFIC DATA SERVICES
Peak Hour: 07:15 AM - 08:15 AM
(303) 216-2439 www.alltrafficdata.net

Peak 15-Minutes: 07:30 AM - 07:45 AM

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval Start Time | CENTENNIAL DR (WCR 46.5¢ENTENNIAL DR (WCR 46.5) |  |  |  |  |  |  |  | PARISH AVE (WCR17) |  |  |  | PARISH AVE (WCR17) |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
|  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru R | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 7:00 AM | 0 | 7 | 3 | 6 | 0 | 3 | 0 | 9 | 0 | 0 | 75 | 4 | 0 | 4 | 19 | 2 | 132 | 501 | 0 | 0 | 0 | 0 |
| 7:15 AM | 0 | 5 | 2 | 11 | 0 | 6 | 1 | 15 | 0 | 0 | 44 | 4 | 0 | 8 | 40 | 0 | 136 | 503 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 8 | 1 | 4 | 0 | 7 | 3 | 10 | 0 | 2 | 51 | 4 | 0 | 4 | 39 | 4 | 137 | 477 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 6 | 2 | 1 | 0 | 1 | 1 | 3 | 0 | 1 | 39 | 3 | 0 | 8 | 27 | 4 | 96 | 472 | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 7 | 0 | 6 | 0 | 4 | 0 | 7 | 0 | 2 | 28 | 3 | 0 | 10 | 61 | 6 | 134 | 458 | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 8 | 0 | 7 | 0 | 5 | 0 | 3 | 0 | 2 | 39 | 2 | 0 | 5 | 37 | 2 | 110 |  | 0 | 0 | 1 | 0 |
| 8:30 AM | 0 | 2 | 1 | 3 | 0 | 0 | 1 | 5 | 0 | 7 | 77 | 3 | 0 | 4 | 26 | 3 | 132 |  | 0 | 0 | 0 | 0 |
| 8:45 AM | 0 | 2 | 2 | 2 | 0 | 3 | 0 | 2 | 0 | 0 | 43 | 0 | 0 | 3 | 20 | 5 | 82 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 45 | 11 | 40 | 0 | 29 | 6 | 54 | 0 | 14 | 396 | 23 | 0 | 46 | 269 | 26 | 959 |  | 0 | 0 | 1 | 0 |
| Peak Hour | 0 | 26 | 5 | 22 | 0 | 18 | 5 | 35 | 0 | 5 | 162 | 14 | 0 | 30 | 167 | 14 | - 50 | 3 | 0 | 0 | 0 | 0 |

Location: 4 MOUNTAIN BLUEBIRD DR \& WCR 46.5 AM
Date: Thursday, October 6, 2022
Peak Hour: 07:15 AM - 08:15 AM
Peak 15-Minutes: 07:15 AM - 07:30 AM

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.

## Traffic Counts

| Interval <br> Start Time | WCR 46.5 <br> Eastbound |  |  |  | WCR 46.5 <br> Westbound |  |  |  |  | MOUNTAIN BLUEBIRD DR <br> Northbound <br> Southbound |  |  |  |  |  |  |  |  |  |  | Rolling Hour | Pedestrian Crossings |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | U-Turn | Left | Thru | Right | U-Turn | Left |  | Thru R | Right | U-Turn | Left | Thru Right | U-Turn | Left |  | Thru |  |  |  |  |  | West | East | South North |
| 7:00 AM | 0 | 1 | 9 | 0 | 0 | 0 |  | 12 | 2 |  |  |  | 0 | 0 | 0 | 0 | O | 0 |  | 24 | 112 | 0 | 0 | 0 |
| 7:15 AM | 0 | 0 | 15 | 0 | 0 | 0 |  | 22 | 1 |  |  |  | 0 | 2 | 2 | 0 | 0 | 0 |  | 40 | 114 | 0 | 0 | 0 |
| 7:30 AM | 0 | 0 | 9 | 0 | 0 | 0 |  | 20 | 2 |  |  |  | 0 | 0 | 0 | 0 | 0 | 0 |  | 31 | 90 | 0 | 0 | 0 |
| 7:45 AM | 0 | 0 | 12 | 0 | 0 | 0 |  | 5 | 0 |  |  |  | 0 | 0 | 0 | 0 | 0 | 0 |  | 17 | 79 | 0 | 0 | 0 |
| 8:00 AM | 0 | 0 | 13 | 0 | 0 | 0 |  | 11 | 1 |  |  |  | 0 | 1 | 1 | 0 | 0 | 0 |  | 26 | 74 | 0 | 0 | 0 |
| 8:15 AM | 0 | 0 | 7 | 0 | 0 | 0 |  | 8 | 1 |  |  |  | 0 | 0 | 0 | 0 | 0 | 0 |  | 16 |  | 0 | 0 | 0 |
| 8:30 AM | 0 | 0 | 9 | 0 | 0 | 0 |  | 6 | 4 |  |  |  | 0 | 1 | 1 | 0 | 0 | 0 |  | 20 |  | 0 | 0 | 0 |
| 8:45 AM | 0 | 0 | 5 | 0 | 0 | 0 |  | 5 | 2 |  |  |  | 0 | 0 | 0 | 0 | 0 | 0 |  | 12 |  | 0 | 0 | 0 |
| Count Total | 0 | 1 | 79 | 0 | 0 | 0 |  | 89 | 13 |  |  |  | 0 |  | 4 | 0 | 0 | 0 | 0 | 186 |  | 0 | 0 | 0 |
| Peak Hour | 0 | 0 | 49 | 0 | 0 | 0 |  | 58 | 4 |  |  |  | 0 |  | 3 |  | 0 |  | 0 | 11 |  | 0 | 0 | 0 |

Location: 1 PARISH AVE (WCR17) \& MOLINAR ST PM
Date: Thursday, October 6, 2022
Peak Hour: 04:45 PM - 05:45 PM
Peak 15-Minutes: 05:00 PM - 05:15 PM

## Peak Hour - Pedestrians/Bicycles on Crosswalk

Peak Hour - All Vehicles



Note: Total study counts contained in parentheses.
Traffic Counts

| Interval <br> Start Time | Eastbound |  |  |  | MOLINAR ST <br> Westbound |  |  |  |  |  | PARISH AVE (WCR17) <br> Northbound |  |  |  | PARISH AVE (WCR17) Southbound |  |  |  |  |  |  | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | U-Turn | Left | Thru | Right | U-Turn | Left |  | Thru |  |  | U-Turn | Left | Thru | Right |  | J-Turn | Left | Thru | Right |  |  |  | West | East | South |  |
| 4:00 PM |  |  |  |  | 0 | 0 |  | 0 |  | 6 | 0 | 0 | 56 | 0 |  | 0 | 2 | 84 | 0 | 0 | 148 | 591 |  | 0 | 0 | 0 |
| 4:15 PM |  |  |  |  | 0 | 1 |  | 0 |  | 1 | 0 | 0 | 56 | 0 |  | 0 | 1 | 81 | 0 | 0 | 140 | 641 |  | 0 | 4 | 0 |
| 4:30 PM |  |  |  |  | 0 | 0 |  | 0 |  | 4 | 0 | 0 | 80 | 0 |  | 0 | 2 | 66 | 0 | 0 | 152 | 692 |  | 0 | 0 | 0 |
| 4:45 PM |  |  |  |  | 0 | 1 |  | 0 |  | 5 | 0 | 0 | 81 | 0 |  | 0 | 1 | 63 | 0 | 0 | 151 | 720 |  | 0 | 5 | 0 |
| 5:00 PM |  |  |  |  | 0 | 0 |  | 0 |  | 6 | 0 | 0 | 94 | 1 |  | 0 | 2 | 95 | 0 | 0 | 198 | 718 |  | 2 | 4 | 0 |
| 5:15 PM |  |  |  |  | 0 | 2 |  | 0 |  | 4 | 0 | 0 | 83 | 0 |  | 0 | 2 | 100 | 0 | 0 | 191 |  |  | 0 | 0 | 0 |
| 5:30 PM |  |  |  |  | 0 | 1 |  | 0 |  | 2 | 0 | 0 | 69 | 0 |  | 0 | 7 | 101 | 0 | 0 | 180 |  |  | 0 | 5 | 0 |
| 5:45 PM |  |  |  |  | 0 | 0 |  | 0 |  | 1 | 0 | 0 | 69 | 0 | 0 | 0 | 1 | 78 | 0 | 0 | 149 |  |  | 0 | 1 | 0 |
| Count Total |  |  |  |  | 0 | 5 | 5 | 0 | 0 | 29 | 0 | 0 | 588 | 1 | 1 | 0 | 18 | 668 |  | 0 | 1,309 |  |  | 2 | 19 | 0 |
| Peak Hour |  |  |  |  | 0 | 4 | 4 | 0 | 0 | 17 | 0 | 0 | 327 | 1 | 1 | 0 | 12 | 359 |  | 0 | 72 | 20 |  | 2 | 14 | 0 |

Location: 2 PARISH AVE (WCR17) \& SETTLER WAY PM
Date: Thursday, October 6, 2022
Peak Hour: 05:00 PM - 06:00 PM
Peak 15-Minutes: 05:15 PM - 05:30 PM


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval | SETTLER WAY <br> Eastbound |  |  |  | SETTLER WAY Westbound |  |  |  | PARISH AVE (WCR17) <br> Northbound |  |  |  | PARISH AVE (WCR17) <br> Southbound |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | Left | Thru |  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 4:00 PM | 0 | 1 | 1 | 0 | 0 | 2 | 0 | 12 | 0 | 1 | 37 | 1 | 0 | 16 | 58 | 0 | 129 | 604 | 0 | 0 | 0 | 0 |
| 4:15 PM | 0 | 1 | 6 | 1 | 0 | 2 | 1 | 9 | 0 | 1 | 46 | 3 | 0 | 12 | 67 | 1 | 150 | 664 | 0 | 2 | 0 | 0 |
| 4:30 PM | 0 | 3 | 1 | 0 | 0 | 14 | 2 | 16 | 0 | 0 | 59 | 6 | 0 | 18 | 47 | 1 | 167 | 704 | 0 | 0 | 0 | 0 |
| 4:45 PM | 0 | 6 | 1 | 0 | 0 | 6 | 1 | 9 | 0 | 3 | 59 | 7 | 0 | 9 | 55 | 2 | 158 | 725 | 2 | 0 | 0 | 0 |
| 5:00 PM | 0 | 6 | 1 | 2 | 0 | 5 | 0 | 14 | 0 | 2 | 71 | 3 | 0 | 20 | 63 | 2 | 189 | 727 | 0 | 0 | 0 | 0 |
| 5:15 PM | 0 | 1 | 0 | 2 | 0 | 3 | 1 | 30 | 0 | 0 | 52 | 8 | 0 | 23 | 68 | 2 | 190 |  | 0 | 0 | 0 | 0 |
| 5:30 PM | 0 | 1 | 0 | 1 | 0 | 3 | 0 | 16 | 0 | 2 | 50 | 4 | 0 | 17 | 91 | 3 | 188 |  | 0 | 0 | 1 | 0 |
| 5:45 PM | 0 | 2 | 0 | 0 | 0 | 3 | 3 | 8 | 0 | 0 | 61 | 6 | 0 | 16 | 59 | 2 | 160 |  | 0 | 4 | 0 | 0 |
| Count Total | 0 | 21 | 10 | 6 | 0 | 38 | 8 | 114 | 0 | 9 | 435 | 38 | 0 | 131 | 508 | 13 | 1,331 |  | 2 | 6 | 1 | 0 |
| Peak Hour | 0 | 10 | 1 | 5 | 0 | 14 | 4 | 68 | 0 | 4 | 234 | 21 | 0 | 76 | 281 | 9 | 72 | 27 | 0 | 4 | 1 | 0 |

Location: 3 PARISH AVE (WCR17) \& CENTENNIAL DR (WCR 46.5) PM
Date: Thursday, October 6, 2022
Peak Hour: 04:45 PM - 05:45 PM
(303) 216-2439 www.alltrafficdata.net

Peak 15-Minutes: 05:30 PM - 05:45 PM

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval <br> Start Time | CENTENNIAL DR (WCR 46.5¢ENTENNIAL DR (WCR 46.5) |  |  |  |  |  |  |  | PARISH AVE (WCR17) |  |  |  | PARISH AVE (WCR17) |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Eastbound |  |  |  | Westbound |  |  |  | Northbound |  |  |  | Southbound |  |  |  |  |  |  |  |  |  |
|  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 4:00 PM | 0 | 0 | 1 | 5 | 1 | 0 | 1 | 8 | 0 | 3 | 33 | 2 | 0 | 5 | 49 | 7 | 115 | 549 | 0 | 0 | 0 | 0 |
| 4:15 PM | 0 | 3 | 0 | 3 | 0 | 5 | 1 | 5 | 0 | 6 | 42 | 8 | 0 | 5 | 60 | 3 | 141 | 599 | 0 | 3 | 2 | 0 |
| 4:30 PM | 0 | 6 | 1 | 3 | 0 | 2 | 0 | 6 | 0 | 7 | 54 | 7 | 0 | 6 | 49 | 8 | 149 | 605 | 0 | 0 | 1 | 0 |
| 4:45 PM | 0 | 5 | 1 | 1 | 0 | 2 | 2 | 11 | 0 | 5 | 52 | 3 | 0 | 6 | 48 | 8 | 144 | 626 | 0 | 0 | 0 | 0 |
| 5:00 PM | 0 | 6 | 1 | 4 | 0 | 5 | 1 | 9 | 0 | 7 | 60 | 2 | 0 | 5 | 61 | 4 | 165 | 621 | 0 | 0 | 0 | 0 |
| 5:15 PM | 0 | 1 | 0 | 5 | 0 | 2 | 0 | 5 | 0 | 8 | 53 | 2 | 0 | 8 | 58 | 5 | 147 |  | 0 | 0 | 0 | 0 |
| 5:30 PM | 0 | 7 | 2 | 3 | 0 | 3 | 0 | 3 | 0 | 8 | 45 | 4 | 0 | 7 | 79 | 9 | 170 |  | 1 | 1 | 0 | 1 |
| 5:45 PM | 0 | 4 | 0 | 1 | 0 | 1 | 1 | 11 | 0 | 6 | 52 | 3 | 0 | 1 | 53 | 6 | 139 |  | 1 | 3 | 0 | 0 |
| Count Total | 0 | 32 | 6 | 25 | 1 | 20 | 6 | 58 | 0 | 50 | 391 | 31 | 0 | 43 | 457 | 50 | 1,170 |  | 2 | 7 | 3 | 1 |
| Peak Hour | 0 | 19 | 4 | 13 | 0 | 12 | 3 | 28 | 0 | 28 | 210 | - 11 | 0 | 26 | 246 | 26 | -62 | 26 | 1 | 1 | 0 | 1 |

Location: 4 MOUNTAIN BLUEBIRD DR \& WCR 46.5 PM
Date: Thursday, October 6, 2022
Peak Hour: 04:00 PM - 05:00 PM
Peak 15-Minutes: 04:45 PM - 05:00 PM

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.

## Traffic Counts

| Interval Start Time | WCR 46.5 <br> Eastbound |  |  |  | WCR 46.5 <br> Westbound |  |  |  | MOUNTAIN BLUEBIRD DR <br> Northbound <br> Southbound |  |  |  |  |  |  |  | Total | Rolling Hour |  | Pedestrian Crossings |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru Right | U-Turn | Left | Thru |  | Right |  |  |  | West | East | South North |
| 4:00 PM | 0 | 0 | 9 | 0 | 0 | 0 | 10 | 4 |  |  |  | 0 | 1 | 0 |  | 0 |  |  | 107 | 0 | 0 | 0 |
| 4:15 PM | 0 | 1 | 12 | 0 | 0 | 0 | 11 | 3 |  |  |  | 0 | 2 | 0 |  | 0 |  |  | 107 | 0 | 0 | 0 |
| 4:30 PM | 0 | 0 | 14 | 0 | 0 | 0 | 7 | 2 |  |  |  | 0 | 1 | 0 |  | 0 |  |  | 100 | 0 | 0 | 0 |
| 4:45 PM | 0 | 0 | 10 | 0 | 0 | 0 | 18 | 1 |  |  |  | 0 | 1 | 0 |  | 0 |  |  | 97 | 0 | 0 | 0 |
| 5:00 PM | 0 | 0 | 8 | 0 | 0 | 0 | 13 | 1 |  |  |  | 0 | 1 | 0 |  | 1 |  |  | 88 | 0 | 0 | 0 |
| 5:15 PM | 0 | 0 | 10 | 0 | 0 | 0 | 7 | 1 |  |  |  | 0 | 4 | 0 |  | 0 |  |  |  | 0 | 0 | 0 |
| 5:30 PM | 0 | 0 | 13 | 0 | 0 | 0 | 6 | 0 |  |  |  | 0 | 2 | 0 |  | 0 |  |  |  | 0 | 0 | 1 |
| 5:45 PM | 0 | 0 | 6 | 0 | 0 | 0 | 14 | 0 |  |  |  | 0 | 1 | 0 |  | 0 |  | 1 |  | 0 | 0 | 0 |
| Count Total | 0 | 1 | 82 | 0 | 0 | 0 | 86 | 12 |  |  |  | 0 | 13 | 0 |  | 1 | 19 | 5 |  | 0 | 0 | 1 |
| Peak Hour | 0 | 1 | 45 | 0 | 0 | 0 | 46 | 10 |  |  |  | 0 | 5 | 5 | 0 | 0 | 0 | 107 |  | 0 | 0 | 0 |

## Appendix B Trip Generation

Alternative: Alternative 1

| Phase: | Open Date: | $4 / 18 / 2023$ |
| :--- | ---: | ---: |
| Project: | Mountain View | Analysis Date: |


|  | Weekday Average Daily Trips |  |  |  | Weekday AM Peak Hour of Adjacent Street Traffic |  |  |  | Weekday PM Peak Hour of Adjacent Street Traffic |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| ITE Land Use | * | Enter | Exit | Total | * | Enter | Exit | Total | * | Enter | Exit | Total |
| 820 SHOPPING PLAZA (ITE 821) | $\checkmark$ | 3714 | 3713 | 7427 | $\checkmark$ | 118 | 72 | 190 | $\checkmark$ | 280 | 291 | 571 |
| 1101000 Sq. Ft. GLA |  |  |  |  |  |  |  |  |  |  |  |  |
| Unadjusted Volume |  | 3714 | 3713 | 7427 |  | 118 | 72 | 190 |  | 280 | 291 | 571 |
| Internal Capture Trips |  | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Pass-By Trips |  | 0 | 0 | 0 |  | 0 | 0 | 0 |  | 97 | 97 | 194 |
| Volume Added to Adjacent Streets |  | 3714 | 3713 | 7427 |  | 118 | 72 | 190 |  | 183 | 194 | 377 |

Total Weekday Average Daily Trips Internal Capture $=0$ Percent
Total Weekday AM Peak Hour of Adjacent Street Traffic Internal Capture $=0$ Percent
Total Weekday PM Peak Hour of Adjacent Street Traffic Internal Capture $=0$ Percent

| Project: Mountain View |  |  |  |  |  |  |  |  |  | Open Date: $4 / 18 / 2023$ <br> Analysis Date: $4 / 18 / 2023$ |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Day / Period | $\begin{aligned} & \text { Total } \\ & \text { Trips } \\ & \hline \end{aligned}$ | $\begin{gathered} \text { Pass-By } \\ \text { Trips } \\ \hline \end{gathered}$ | $\begin{gathered} \text { Avg } \\ \text { Rate } \\ \hline \end{gathered}$ | $\begin{gathered} \text { Min } \\ \text { Rate } \\ \hline \end{gathered}$ | $\begin{array}{r} \text { Max } \\ \text { Rate } \\ \hline \end{array}$ | $\begin{array}{r} \mathrm{Std} \\ \mathrm{Dev} \\ \hline \end{array}$ | $\begin{gathered} \text { Avg } \\ \text { Size } \\ \hline \end{gathered}$ | \% Enter | $\begin{gathered} \% \\ \text { Exit } \\ \hline \end{gathered}$ | $\begin{aligned} & \text { Use } \\ & \text { Eq. } \end{aligned}$ | Equation | R2 |
| Weekday Average Daily Trips <br> Source: ITE 11-821-Custom | 7427 | 0 | 67.52 | 43.29 | 91.06 | 19.25 | 59 | 50 | 50 | False | $\mathrm{T}=0.0(\mathrm{X})+0.0$ | 0 |
| Weekday AM Peak Hour of Adjacent Street Traffic <br> Source: ITE 11-821-Custom | 190 | 0 | 1.73 | 0.29 | 3.77 | 1.06 | 67 | 62 | 38 | False | $\mathrm{T}=0.0(\mathrm{X})+0.0$ | 0 |
| Weekday PM Peak Hour of Adjacent Street Traffic Source: ITE 11-821-Custom | 571 | 194 | 5.19 | 2.55 | 15.31 | 2.28 | 79 | 49 | 51 | False | $\mathrm{T}=0.0(\mathrm{X})+0.0$ | 0 |

## Appendix C Synchro Reports



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.4 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{1}$ | $\mathbf{7}$ | $\mathbf{F}$ |  |  | $\mathbf{-}$ |
| Traffic Vol, veh/h | 0 | 17 | 255 | 0 | 10 | 274 |
| Future Vol, veh/h | 0 | 17 | 255 | 0 | 10 | 274 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 90 | 0 | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 85 | 85 | 66 | 66 | 69 | 69 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 20 | 386 | 0 | 14 | 397 |



|  | 4 | $\rightarrow$ |  | 6 |  | 4 | 4 | 4 | \% | ( | $\frac{1}{7}$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | 4 | 「 | ${ }^{7}$ | 4 | 「 |
| Traffic Volume (vph) | 8 | 3 | 4 | 7 | 1 | 48 | 3 | 215 | 8 | 49 | 210 | 21 |
| Future Volume (vph) | 8 | 3 | 4 | 7 | 1 | 48 | 3 | 215 | 8 | 49 | 210 | 21 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 200 |  | 0 | 80 |  | 0 | 180 |  | 230 | 140 |  | 130 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 1 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.917 |  |  | 0.852 |  |  |  | 0.850 |  |  | 0.850 |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1770 | 1708 | 0 | 1770 | 1587 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 1770 | 1708 | 0 | 1770 | 1587 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 626 |  |  | 658 |  |  | 806 |  |  | 578 |  |
| Travel Time (s) |  | 14.2 |  |  | 15.0 |  |  | 15.7 |  |  | 11.3 |  |
| Peak Hour Factor | 0.79 | 0.79 | 0.79 | 0.70 | 0.70 | 0.70 | 0.76 | 0.76 | 0.76 | 0.71 | 0.71 | 0.71 |
| Adj. Flow (vph) | 10 | 4 | 5 | 10 | 1 | 69 | 4 | 283 | 11 | 69 | 296 | 30 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 10 | 9 | 0 | 10 | 70 | 0 | 4 | 283 | 11 | 69 | 296 | 30 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |

## Intersection Summary

```
Area Type: Other
```

Control Type: Unsignalized
Intersection Capacity Utilization 31.3\% ICU Level of Service A
Analysis Period (min) 15

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 2.2 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{1}$ | $\uparrow$ |  | \% | $\uparrow$ |  | ${ }^{*}$ | 4 | 「 | ${ }^{*}$ | 4 | 「 |
| Traffic Vol, veh/h | 8 | 3 | 4 | 7 | 1 | 48 | 3 | 215 | 8 | 49 | 210 | 21 |
| Future Vol, veh/h | 8 | 3 | 4 | 7 | 1 | 48 | 3 | 215 | 8 | 49 | 210 | 21 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | 200 | - | - | 80 | - | - | 180 | - | 230 | 140 | - | 130 |
| Veh in Median Storage, \# | \# - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 79 | 79 | 79 | 70 | 70 | 70 | 76 | 76 | 76 | 71 | 71 | 71 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 10 | 4 | 5 | 10 | 1 | 69 | 4 | 283 | 11 | 69 | 296 | 30 |



|  | 4 | $\rightarrow$ |  | 7 |  |  | 4 | $\uparrow$ | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | $\uparrow$ |  | \% | $\uparrow$ | F | ${ }^{7}$ | $\uparrow$ | F |
| Traffic Volume (vph) | 26 | 5 | 22 | 18 | 5 | 35 | 5 | 162 | 14 | 30 | 167 | 14 |
| Future Volume (vph) | 26 | 5 | 22 | 18 | 5 | 35 | 5 | 162 | 14 | 30 | 167 | 14 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 |  | 0 | 0 |  | 0 | 240 |  | 290 | 260 |  | 200 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 1 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.944 |  |  | 0.918 |  |  |  | 0.850 |  |  | 0.850 |
| Flt Protected |  | 0.976 |  |  | 0.985 |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 1716 | 0 | 0 | 1684 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Flt Permitted |  | 0.976 |  |  | 0.985 |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 0 | 1716 | 0 | 0 | 1684 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 516 |  |  | 716 |  |  | 680 |  |  | 806 |  |
| Travel Time (s) |  | 11.7 |  |  | 16.3 |  |  | 13.2 |  |  | 15.7 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.67 | 0.67 | 0.67 | 0.72 | 0.72 | 0.72 | 0.69 | 0.69 | 0.69 |
| Adj. Flow (vph) | 33 | 6 | 28 | 27 | 7 | 52 | 7 | 225 | 19 | 43 | 242 | 20 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 67 | 0 | 0 | 86 | 0 | 7 | 225 | 19 | 43 | 242 | 20 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(t) |  | 0 |  |  | 0 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |

## Intersection Summary

```
Area Type: Other
```

Control Type: Unsignalized
Intersection Capacity Utilization 27.1\% ICU Level of Service A
Analysis Period (min) 15



|  | 4 | $\rightarrow$ |  |  | ( | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow$ | $\uparrow$ |  | ${ }^{1}$ | 「 |
| Traffic Volume (vph) | 0 | 49 | 58 | 4 | 3 | 0 |
| Future Volume (vph) | 0 | 49 | 58 | 4 | 3 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 |  |  | 0 | 90 | 0 |
| Storage Lanes | 0 |  |  | 0 | 1 | 1 |
| Taper Length (ft) | 25 |  |  |  | 25 |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  |  | 0.991 |  |  |  |
| Flt Protected |  |  |  |  | 0.950 |  |
| Satd. Flow (prot) | 0 | 1863 | 1846 | 0 | 1770 | 1863 |
| Flt Permitted |  |  |  |  | 0.950 |  |
| Satd. Flow (perm) | 0 | 1863 | 1846 | 0 | 1770 | 1863 |
| Link Speed (mph) |  | 30 | 30 |  | 30 |  |
| Link Distance (ft) |  | 716 | 677 |  | 714 |  |
| Travel Time (s) |  | 16.3 | 15.4 |  | 16.2 |  |
| Peak Hour Factor | 0.82 | 0.82 | 0.70 | 0.70 | 0.38 | 0.38 |
| Adj. Flow (vph) | 0 | 60 | 83 | 6 | 8 | 0 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 60 | 89 | 0 | 8 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Left | Right | Left | Right |
| Median Width(ft) |  | 0 | 0 |  | 12 |  |
| Link Offset(ft) |  | 0 | 0 |  | 0 |  |
| Crosswalk Width(ft) |  | 16 | 16 |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  |  | 9 | 15 | 9 |
| Sign Control |  | Free | Free |  | Stop |  |


| Intersection Summary |  |
| :--- | :--- |
| Area Type: Other |  |
| Control Type: Unsignalized |  |
| Intersection Capacity Utilization $13.3 \%$ |  |
| Analysis Period ( $\min$ ) 15 |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.5 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow$ | $\uparrow$ |  | I | $\mathbf{7}$ |
| Traffic Vol, veh/h | 0 | 49 | 58 | 4 | 3 | 0 |
| Future Vol, veh/h | 0 | 49 | 58 | 4 | 3 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 90 | 0 |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 82 | 82 | 70 | 70 | 38 | 38 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 60 | 83 | 6 | 8 | 0 |




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.5 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{1}$ | $\mathbf{7}$ | $\mathbf{1}$ |  |  | $\mathbf{-}$ |
| Traffic Vol, veh/h | 4 | 17 | 327 | 1 | 12 | 359 |
| Future Vol, veh/h | 4 | 17 | 327 | 1 | 12 | 359 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 90 | 0 | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 89 | 89 | 89 | 89 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 4 | 18 | 367 | 1 | 13 | 403 |



|  | 4 | $\rightarrow$ |  | 7 | 4 | 4 | 4 | $\dagger$ | $p$ | ( | 1 | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | 4 | 「 | ${ }^{7}$ | 4 | 「 |
| Traffic Volume (vph) | 10 | 1 | 5 | 14 | 4 | 68 | 4 | 234 | 21 | 76 | 281 | 9 |
| Future Volume (vph) | 10 | 1 | 5 | 14 | 4 | 68 | 4 | 234 | 21 | 76 | 281 | 9 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 200 |  | 0 | 80 |  | 0 | 180 |  | 230 | 140 |  | 130 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 1 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.871 |  |  | 0.858 |  |  |  | 0.850 |  |  | 0.850 |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1770 | 1622 | 0 | 1770 | 1598 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 1770 | 1622 | 0 | 1770 | 1598 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 626 |  |  | 658 |  |  | 806 |  |  | 578 |  |
| Travel Time (s) |  | 14.2 |  |  | 15.0 |  |  | 15.7 |  |  | 11.3 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.74 | 0.74 | 0.74 | 0.89 | 0.89 | 0.89 | 0.82 | 0.82 | 0.82 |
| Adj. Flow (vph) | 13 | 1 | 6 | 19 | 5 | 92 | 4 | 263 | 24 | 93 | 343 | 11 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 13 | 7 | 0 | 19 | 97 | 0 | 4 | 263 | 24 | 93 | 343 | 11 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 35.6\% ICU Level of Service A

Analysis Period (min) 15

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 2.9 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{1}$ | $\hat{\beta}$ |  | ${ }^{7}$ | $\hat{\sigma}$ |  | ${ }^{*}$ | 4 | 「 | ${ }^{1}$ | 4 | 「 |
| Traffic Vol, veh/h | 10 | 1 | 5 | 14 | 4 | 68 | 4 | 234 | 21 | 76 | 281 | 9 |
| Future Vol, veh/h | 10 | 1 | 5 | 14 | 4 | 68 | 4 | 234 | 21 | 76 | 281 | 9 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | 200 | - | - | 80 | - | - | 180 | - | 230 | 140 | - | 130 |
| Veh in Median Storage, \# | \# - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 74 | 74 | 74 | 89 | 89 | 89 | 82 | 82 | 82 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 13 | 1 | 6 | 19 | 5 | 92 | 4 | 263 | 24 | 93 | 343 | 11 |



|  | 4 | $\rightarrow$ |  | $\dagger$ |  |  | 4 | $\uparrow$ | 7 |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | \$ |  | \% | $\uparrow$ | F | ${ }^{7}$ | $\uparrow$ | F |
| Traffic Volume (vph) | 19 | 4 | 13 | 12 | 3 | 28 | 28 | 210 | 11 | 26 | 246 | 26 |
| Future Volume (vph) | 19 | 4 | 13 | 12 | 3 | 28 | 28 | 210 | 11 | 26 | 246 | 26 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 |  | 0 | 0 |  | 0 | 240 |  | 290 | 260 |  | 200 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 1 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.951 |  |  | 0.913 |  |  |  | 0.850 |  |  | 0.850 |
| Flt Protected |  | 0.974 |  |  | 0.986 |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 1725 | 0 | 0 | 1677 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Flt Permitted |  | 0.974 |  |  | 0.986 |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 0 | 1725 | 0 | 0 | 1677 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 516 |  |  | 716 |  |  | 680 |  |  | 806 |  |
| Travel Time (s) |  | 11.7 |  |  | 16.3 |  |  | 13.2 |  |  | 15.7 |  |
| Peak Hour Factor | 0.75 | 0.75 | 0.75 | 0.82 | 0.82 | 0.82 | 0.94 | 0.94 | 0.94 | 0.78 | 0.78 | 0.78 |
| Adj. Flow (vph) | 25 | 5 | 17 | 15 | 4 | 34 | 30 | 223 | 12 | 33 | 315 | 33 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 47 | 0 | 0 | 53 | 0 | 30 | 223 | 12 | 33 | 315 | 33 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 0 |  |  | 0 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |

## Intersection Summary

```
Area Type: Other
```

Control Type: Unsignalized
Intersection Capacity Utilization 30.0\% ICU Level of Service A
Analysis Period (min) 15



| Minor Lane/Major Mvmt | NBL | NBT | NBR EBLn1WBLn1 | SBL | SBT | SBR |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1211 | - | - | 414 | 545 | 1332 | - |



| Intersection Summary $\quad$ Other |  |
| :--- | :--- |
| Area Type: |  |
| Control Type: Unsignalized |  |
| Intersection Capacity Utilization 13.3\% | ICU Level of Service A |
| Analysis Period (min) 15 |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.7 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | -1 | $\mathbf{F}$ |  | 1 | $\mathbf{7}$ |
| Traffic Vol, veh/h | 1 | 45 | 46 | 10 | 5 | 0 |
| Future Vol, veh/h | 1 | 45 | 46 | 10 | 5 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 90 | 0 |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 82 | 82 | 74 | 74 | 56 | 56 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 55 | 62 | 14 | 9 | 0 |



|  | 7 | 4 | 4 | $p$ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ${ }^{17}$ | F | f |  |  | $\uparrow$ |
| Trafic Volume (vph) | 17 | 55 | 282 | 5 | 23 | 295 |
| Future Volume (vph) | 17 | 55 | 282 | 5 | 23 | 295 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 90 | 0 |  | 0 | 0 |  |
| Storage Lanes | 1 | 1 |  | 0 | 0 |  |
| Taper Length (ft) | 25 |  |  |  | 25 |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Fit |  | 0.850 | 0.998 |  |  |  |
| Flt Protected | 0.950 |  |  |  |  | 0.996 |
| Satd. Flow (prot) | 1770 | 1583 | 1859 | 0 | 0 | 1855 |
| Flt Permitted | 0.950 |  |  |  |  | 0.996 |
| Satd. Flow (perm) | 1770 | 1583 | 1859 | 0 | 0 | 1855 |
| Link Speed (mph) | 30 |  | 35 |  |  | 35 |
| Link Distance (ft) | 725 |  | 578 |  |  | 580 |
| Travel Time (s) | 16.5 |  | 11.3 |  |  | 11.3 |
| Peak Hour Factor | 0.78 | 0.79 | 0.88 | 0.78 | 0.78 | 0.89 |
| Adj. Flow (vph) | 22 | 70 | 320 | 6 | 29 | 331 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 22 | 70 | 326 | 0 | 0 | 360 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(ft) | 12 |  | 12 |  |  | 12 |
| Link Offset(ft) | 0 |  | 0 |  |  | 0 |
| Crosswalk Width(ft) | 16 |  | 16 |  |  | 16 |
| Two way Left Turn Lane |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 | 9 |  | 9 | 15 |  |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |
| Control Type: Unsignalized |  |  |  |  |  |  |
| Intersection Capacity Utilization 44.5\% ICU Level of Service A |  |  |  |  |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.7 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | 7 | $\mathbf{r}$ | $\mathbf{b}$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 17 | 55 | 282 | 5 | 23 | 295 |
| Future Vol, veh/h | 17 | 55 | 282 | 5 | 23 | 295 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 90 | 0 | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 78 | 79 | 88 | 78 | 78 | 89 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 22 | 70 | 320 | 6 | 29 | 331 |



|  | 4 | $\rightarrow$ |  | 7 | 4 | 4 | 4 | $\dagger$ | $p$ | ( | 1 | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | ${ }^{*}$ | $\uparrow$ |  | ${ }^{7}$ | 4 | 「' | \% | 4 | F |
| Traffic Volume (vph) | 8 | 3 | 4 | 11 | 1 | 62 | 3 | 233 | 9 | 56 | 240 | 22 |
| Future Volume (vph) | 8 | 3 | 4 | 11 | 1 | 62 | 3 | 233 | 9 | 56 | 240 | 22 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 200 |  | 0 | 80 |  | 0 | 180 |  | 230 | 140 |  | 130 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 1 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.917 |  |  | 0.852 |  |  |  | 0.850 |  |  | 0.850 |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1770 | 1708 | 0 | 1770 | 1587 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 1770 | 1708 | 0 | 1770 | 1587 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 626 |  |  | 658 |  |  | 806 |  |  | 578 |  |
| Travel Time (s) |  | 14.2 |  |  | 15.0 |  |  | 15.7 |  |  | 11.3 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.79 | 0.78 | 0.88 | 0.78 | 0.79 | 0.88 | 0.78 |
| Adj. Flow (vph) | 10 | 4 | 5 | 14 | 1 | 78 | 4 | 265 | 12 | 71 | 273 | 28 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 10 | 9 | 0 | 14 | 79 | 0 | 4 | 265 | 12 | 71 | 273 | 28 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |

## Intersection Summary

```
Area Type: Other
```

Control Type: Unsignalized
Intersection Capacity Utilization 33.2\% ICU Level of Service A
Analysis Period (min) 15



[^0]|  | $\stackrel{*}{ }$ |  |  | 7 |  |  |  | 4 | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | \$ |  | \% | 4 | F' | ${ }^{7}$ | 4 | F |
| Trafic Volume (vph) | 28 | 5 | 23 | 23 | 5 | 37 | 5 | 178 | 16 | 32 | 198 | 15 |
| Future Volume (vph) | 28 | 5 | 23 | 23 | 5 | 37 | 5 | 178 | 16 | 32 | 198 | 15 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 |  | 0 | 0 |  | 0 | 240 |  | 290 | 260 |  | 200 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 1 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.945 |  |  | 0.923 |  |  |  | 0.850 |  |  | 0.850 |
| FIt Protected |  | 0.975 |  |  | 0.983 |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 1716 | 0 | 0 | 1690 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| FIt Permitted |  | 0.975 |  |  | 0.983 |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 0 | 1716 | 0 | 0 | 1690 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 516 |  |  | 716 |  |  | 680 |  |  | 806 |  |
| Travel Time (s) |  | 11.7 |  |  | 16.3 |  |  | 13.2 |  |  | 15.7 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.86 | 0.78 | 0.78 | 0.87 | 0.78 |
| Adj. Flow (vph) | 36 | 6 | 29 | 29 | 6 | 47 | 6 | 207 | 21 | 41 | 228 | 19 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 71 | 0 | 0 | 82 | 0 | 6 | 207 | 21 | 41 | 228 | 19 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 0 |  |  | 0 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |

## Intersection Summary

```
Area Type: Other
```

Control Type: Unsignalized
Intersection Capacity Utilization 28.7\% ICU Level of Service A
Analysis Period (min) 15

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 3.5 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | \& |  | ${ }^{1}$ | 4 | F | ${ }^{*}$ | 4 | 「 |
| Traffic Vol, veh/h | 28 | 5 | 23 | 23 | 5 | 37 | 5 | 178 | 16 | 32 | 198 | 15 |
| Future Vol, veh/h | 28 | 5 | 23 | 23 | 5 | 37 | 5 | 178 | 16 | 32 | 198 | 15 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | 240 | - | 290 | 260 | - | 200 |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 86 | 78 | 78 | 87 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mumt Flow | 36 | 6 | 29 | 29 | 6 | 47 | 6 | 207 | 21 | 41 | 228 | 19 |



| Minor Lane/Major Mvmt | NBL | NBT | NBR EBLn1WBLn1 | SBL | SBT | SBR |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1319 | - | - | 505 | 579 | 1340 | - |

[^1]|  | $\stackrel{*}{*}$ | $\rightarrow$ |  |  | 4 |  | 4 | $\dagger$ | 7 |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ${ }_{4}$ |  |  | ¢ |  | 7 | F |  | \% | $\uparrow$ |  |
| Traffic Volume (vph) | 1 | 52 | 0 | 0 | 62 | 6 | 0 | 0 | 0 | 11 | 0 | 4 |
| Future Volume (vph) | 1 | 52 | 0 | 0 | 62 | 6 | 0 | 0 | 0 | 11 | 0 | 4 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 |  | 0 | 0 |  | 0 | 90 |  | 0 | 90 |  | 0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  |  |  |  | 0.987 |  |  |  |  |  | 0.850 |  |
| Flt Protected |  | 0.999 |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 1861 | 0 | 0 | 1839 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Flt Permitted |  | 0.999 |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 0 | 1861 | 0 | 0 | 1839 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (ft) |  | 716 |  |  | 677 |  |  | 727 |  |  | 714 |  |
| Travel Time (s) |  | 16.3 |  |  | 15.4 |  |  | 16.5 |  |  | 16.2 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.79 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 |
| Adj. Flow (vph) | 1 | 67 | 0 | 0 | 78 | 8 | 0 | 0 | 0 | 14 | 0 | 5 |
| Shared Lane Trafic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 68 | 0 | 0 | 86 | 0 | 0 | 0 | 0 | 14 | 5 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 0 |  |  | 0 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 13.6\% ICU Level of Service A
Analysis Period (min) 15



[^2]|  | 7 |  | $\dagger$ | $p$ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | \% | F | $\dagger$ |  |  | $\uparrow$ |
| Traffic Volume (vph) | 14 | 41 | 354 | 19 | 52 | 393 |
| Future Volume (vph) | 14 | 41 | 354 | 19 | 52 | 393 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 90 | 0 |  | 0 | 0 |  |
| Storage Lanes | 1 | 1 |  | 0 | 0 |  |
| Taper Length (ft) | 25 |  |  |  | 25 |  |
| Lane Utill. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.850 | 0.992 |  |  |  |
| Flt Protected | 0.950 |  |  |  |  | 0.993 |
| Satd. Flow (prot) | 1770 | 1583 | 1848 | 0 | 0 | 1850 |
| Flt Permitted | 0.950 |  |  |  |  | 0.993 |
| Satd. Flow (perm) | 1770 | 1583 | 1848 | 0 | 0 | 1850 |
| Link Speed (mph) | 30 |  | 35 |  |  | 35 |
| Link Distance (ft) | 725 |  | 578 |  |  | 580 |
| Travel Time (s) | 16.5 |  | 11.3 |  |  | 11.3 |
| Peak Hour Factor | 0.78 | 0.78 | 0.89 | 0.78 | 0.78 | 0.90 |
| Adj. Flow (vph) | 18 | 53 | 398 | 24 | 67 | 437 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 18 | 53 | 422 | 0 | 0 | 504 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(ft) | 12 |  | 12 |  |  | 12 |
| Link Offset(ft) | 0 |  | 0 |  |  | 0 |
| Crosswalk Width(ft) | 16 |  | 16 |  |  | 16 |
| Two way Left Turn Lane |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 | 9 |  | 9 | 15 |  |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |
| Control Type: Unsignalized |  |  |  |  |  |  |
| Intersection Capacity Utilization 56.7\%Analysis Period (min) 15 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.5 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | 1 | $\mathbf{r}$ | $\mathbf{F}$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 14 | 41 | 354 | 19 | 52 | 393 |
| Future Vol, veh/h | 14 | 41 | 354 | 19 | 52 | 393 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 90 | 0 | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 78 | 78 | 89 | 78 | 78 | 90 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 18 | 53 | 398 | 24 | 67 | 437 |




## Intersection Summary

```
Area Type: Other
```

Control Type: Unsignalized
Intersection Capacity Utilization 37.2\% ICU Level of Service A
Analysis Period (min) 15

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 3.2 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | $\hat{\beta}$ |  | \% | $\hat{\sigma}$ |  | ${ }^{*}$ | 4 | 「 | ${ }^{7}$ | 4 | 7 |
| Traffic Vol, veh/h | 11 | 1 | 5 | 17 | 4 | 79 | 4 | 266 | 26 | 93 | 308 | 10 |
| Future Vol, veh/h | 11 | 1 | 5 | 17 | 4 | 79 | 4 | 266 | 26 | 93 | 308 | 10 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | 200 | - | - | 80 | - | - | 180 | - | 230 | 140 | - | 130 |
| Veh in Median Storage, \# | \# - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 81 | 78 | 88 | 78 | 82 | 89 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 14 | 1 | 6 | 22 | 5 | 98 | 5 | 302 | 33 | 113 | 346 | 13 |



[^3]|  | $\stackrel{*}{ }$ |  |  | 7 |  |  |  | 4 | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | \$ |  | ${ }^{7}$ | 4 | F' | ${ }^{7}$ | $\uparrow$ | F |
| Trafic Volume (vph) | 20 | 4 | 14 | 15 | 3 | 30 | 30 | 245 | 16 | 28 | 273 | 28 |
| Future Volume (vph) | 20 | 4 | 14 | 15 | 3 | 30 | 30 | 245 | 16 | 28 | 273 | 28 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 |  | 0 | 0 |  | 0 | 240 |  | 290 | 260 |  | 200 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 1 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.950 |  |  | 0.916 |  |  |  | 0.850 |  |  | 0.850 |
| FIt Protected |  | 0.974 |  |  | 0.985 |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 1724 | 0 | 0 | 1681 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| FIt Permitted |  | 0.974 |  |  | 0.985 |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 0 | 1724 | 0 | 0 | 1681 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 516 |  |  | 716 |  |  | 680 |  |  | 806 |  |
| Travel Time (s) |  | 11.7 |  |  | 16.3 |  |  | 13.2 |  |  | 15.7 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.88 | 0.78 | 0.78 | 0.88 | 0.78 |
| Adj. Flow (vph) | 26 | 5 | 18 | 19 | 4 | 38 | 38 | 278 | 21 | 36 | 310 | 36 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 49 | 0 | 0 | 61 | 0 | 38 | 278 | 21 | 36 | 310 | 36 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 0 |  |  | 0 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 31.4\% ICU Level of Service A
Analysis Period (min) 15



[^4]|  | $\stackrel{*}{ }$ |  |  | 1 |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | $\uparrow$ |  | \% | $\hat{1}$ |  | ${ }^{7}$ | $\hat{1}$ |  |
| Traffic Volume (vph) | 5 | 48 | 0 | 0 | 49 | 20 | 0 | 0 | 0 | 10 | 0 | 2 |
| Future Volume (vph) | 5 | 48 | 0 | 0 | 49 | 20 | 0 | 0 | 0 | 10 | 0 | 2 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 |  | 0 | 0 |  | 0 | 90 |  | 0 | 90 |  | 0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Utill. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  |  |  |  | 0.961 |  |  |  |  |  | 0.850 |  |
| Flt Protected |  | 0.996 |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 1855 | 0 | 0 | 1790 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Flt Permitted |  | 0.996 |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 0 | 1855 | 0 | 0 | 1790 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (ft) |  | 716 |  |  | 677 |  |  | 727 |  |  | 714 |  |
| Travel Time (s) |  | 16.3 |  |  | 15.4 |  |  | 16.5 |  |  | 16.2 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 |
| Adj. Flow (vph) | 6 | 62 | 0 | 0 | 63 | 26 | 0 | 0 | 0 | 13 | 0 | 3 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 68 | 0 | 0 | 89 | 0 | 0 | 0 | 0 | 13 | 3 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 0 |  |  | 0 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 60 | 60 |  | 9 | 60 |  | 60 | 15 |  | 9 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 16.7\% ICU Level of Service A
Analysis Period (min) 15




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.5 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | 7 | $\mathbf{r}$ | $\mathbf{b}$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 17 | 55 | 282 | 5 | 23 | 295 |
| Future Vol, veh/h | 17 | 55 | 325 | 5 | 23 | 366 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 90 | 0 | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 78 | 79 | 89 | 78 | 78 | 90 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 22 | 70 | 365 | 6 | 29 | 407 |



[^5]|  | 4 | $\rightarrow$ |  | 6 |  | 4 | 4 | 4 | \% | $\pm$ | $\frac{1}{7}$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | 4 | 「 | ${ }^{7}$ | 4 | 「 |
| Traffic Volume (vph) | 8 | 3 | 4 | 11 | 1 | 62 | 3 | 233 | 9 | 56 | 240 | 22 |
| Future Volume (vph) | 8 | 3 | 4 | 22 | 1 | 105 | 3 | 233 | 27 | 127 | 240 | 22 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 200 |  | 0 | 80 |  | 0 | 180 |  | 230 | 140 |  | 130 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 1 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.917 |  |  | 0.851 |  |  |  | 0.850 |  |  | 0.850 |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1770 | 1708 | 0 | 1770 | 1585 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 1770 | 1708 | 0 | 1770 | 1585 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 626 |  |  | 658 |  |  | 806 |  |  | 578 |  |
| Travel Time (s) |  | 14.2 |  |  | 15.0 |  |  | 15.7 |  |  | 11.3 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.83 | 0.78 | 0.89 | 0.78 | 0.84 | 0.89 | 0.78 |
| Adj. Flow (vph) | 10 | 4 | 5 | 28 | 1 | 127 | 4 | 262 | 35 | 151 | 270 | 28 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 10 | 9 | 0 | 28 | 128 | 0 | 4 | 262 | 35 | 151 | 270 | 28 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |

## Intersection Summary

```
Area Type: Other
```

Control Type: Unsignalized
Intersection Capacity Utilization 33.2\% ICU Level of Service A
Analysis Period (min) 15

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 3.9 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{1}$ | $\uparrow$ |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{*}$ | 4 | F゙ | ${ }^{1}$ | 4 | 「 |
| Traffic Vol, veh/h | 8 | 3 | 4 | 11 | 1 | 62 | 3 | 233 | 9 | 56 | 240 | 22 |
| Future Vol, veh/h | 8 | 3 | 4 | 22 | 1 | 105 | 3 | 233 | 27 | 127 | 240 | 22 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | 200 | - | - | 80 | - | - | 180 | - | 230 | 140 | - | 130 |
| Veh in Median Storage, \# | \# - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 83 | 78 | 89 | 78 | 84 | 89 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 10 | 4 | 5 | 28 | 1 | 127 | 4 | 262 | 35 | 151 | 270 | 28 |



[^6]|  | $\stackrel{*}{ }$ |  |  | 7 |  |  |  | 4 | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | \$ |  | \% | 4 | F' | ${ }^{7}$ | $\uparrow$ | F |
| Trafic Volume (vph) | 28 | 5 | 23 | 23 | 5 | 37 | 5 | 178 | 16 | 32 | 198 | 15 |
| Future Volume (vph) | 28 | 5 | 23 | 34 | 5 | 37 | 5 | 196 | 34 | 32 | 209 | 15 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 |  | 0 | 0 |  | 0 | 240 |  | 290 | 260 |  | 200 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 1 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.945 |  |  | 0.935 |  |  |  | 0.850 |  |  | 0.850 |
| FIt Protected |  | 0.975 |  |  | 0.978 |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 1716 | 0 | 0 | 1703 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| FIt Permitted |  | 0.975 |  |  | 0.978 |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 0 | 1716 | 0 | 0 | 1703 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 516 |  |  | 716 |  |  | 680 |  |  | 806 |  |
| Travel Time (s) |  | 11.7 |  |  | 16.3 |  |  | 13.2 |  |  | 15.7 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.87 | 0.78 | 0.78 | 0.87 | 0.78 |
| Adj. Flow (vph) | 36 | 6 | 29 | 44 | 6 | 47 | 6 | 225 | 44 | 41 | 240 | 19 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 71 | 0 | 0 | 97 | 0 | 6 | 225 | 44 | 41 | 240 | 19 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 0 |  |  | 0 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |

## Intersection Summary

```
Area Type: Other
```

Control Type: Unsignalized
Intersection Capacity Utilization 28.7\% ICU Level of Service A
Analysis Period (min) 15

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 3.6 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow$ |  |  | \& |  | ${ }^{7}$ | 4 | 7' | ${ }^{1 /}$ | 4 | 7 |
| Traffic Vol, veh/h | 28 | 5 | 23 | 23 | 5 | 37 | 5 | 178 | 16 | 32 | 198 | 15 |
| Future Vol, veh/h | 28 | 5 | 23 | 34 | 5 | 37 | 5 | 196 | 34 | 32 | 209 | 15 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | 240 | - | 290 | 260 | - | 200 |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 87 | 78 | 78 | 87 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 36 | 6 | 29 | 44 | 6 | 47 | 6 | 225 | 44 | 41 | 240 | 19 |



[^7]|  | $\rangle$ |  |  | 7 |  |  | 4 | $\dagger$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | $\dagger$ |  | \% | $\hat{F}$ |  | ${ }_{1}$ | $\hat{F}$ |  |
| Traffic Volume (vph) | 1 | 52 | 0 | 0 | 62 | 6 | 0 | 0 | 0 | 11 | 0 | 4 |
| Future Volume (vph) | 19 | 52 | 0 | 0 | 62 | 18 | 0 | 0 | 0 | 18 | 0 | 15 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 |  | 0 | 0 |  | 0 | 90 |  | 0 | 90 |  | 0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  |  |  |  | 0.969 |  |  |  |  |  | 0.850 |  |
| Flt Protected |  | 0.987 |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 1839 | 0 | 0 | 1805 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Flt Permitted |  | 0.987 |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 0 | 1839 | 0 | 0 | 1805 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (ft) |  | 716 |  |  | 677 |  |  | 727 |  |  | 714 |  |
| Travel Time (s) |  | 16.3 |  |  | 15.4 |  |  | 16.5 |  |  | 16.2 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.79 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 |
| Adj. Flow (vph) | 24 | 67 | 0 | 0 | 78 | 23 | 0 | 0 | 0 | 23 | 0 | 19 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 91 | 0 | 0 | 101 | 0 | 0 | 0 | 0 | 23 | 19 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 0 |  |  | 0 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 13.6\% ICU Level of Service A
Analysis Period (min) 15

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 2.5 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \& |  |  | \& |  | ${ }^{1}$ | $\uparrow$ |  | ${ }^{1}$ | $\uparrow$ |  |
| Traffic Vol, veh/h | 1 | 52 | 0 | 0 | 62 | 6 | 0 | 0 | 0 | 11 | 0 | 4 |
| Future Vol, veh/h | 19 | 52 | 0 | 0 | 62 | 18 | 0 | 0 | 0 | 18 | 0 | 15 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control Fr | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | 90 | - | - | 90 | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 79 | 78 | 78 | 78 | 78 | 78 | 78 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 24 | 67 | 0 | 0 | 78 | 23 | 0 | 0 | 0 | 23 | 0 | 19 |



|  |  | 4 |  |  |  | $\frac{1}{1}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ${ }^{7}$ | 「 | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Volume (vph) | 14 | 41 | 354 | 19 | 52 | 393 |
| Future Volume (vph) | 14 | 41 | 529 | 19 | 52 | 561 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 90 | 0 |  | 0 | 0 |  |
| Storage Lanes | 1 | 1 |  | 0 | 0 |  |
| Taper Length (ft) | 25 |  |  |  | 25 |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.850 | 0.995 |  |  |  |
| Flt Protected | 0.950 |  |  |  |  | 0.995 |
| Satd. Flow (prot) | 1770 | 1583 | 1853 | 0 | 0 | 1853 |
| Flt Permitted | 0.950 |  |  |  |  | 0.995 |
| Satd. Flow (perm) | 1770 | 1583 | 1853 | 0 | 0 | 1853 |
| Link Speed (mph) | 30 |  | 35 |  |  | 35 |
| Link Distance (ft) | 725 |  | 578 |  |  | 580 |
| Travel Time (s) | 16.5 |  | 11.3 |  |  | 11.3 |
| Peak Hour Factor | 0.78 | 0.78 | 0.92 | 0.78 | 0.78 | 0.92 |
| Adj. Flow (vph) | 18 | 53 | 575 | 24 | 67 | 610 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 18 | 53 | 599 | 0 | 0 | 677 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(ft) | 12 |  | 12 |  |  | 12 |
| Link Offset(ft) | 0 |  | 0 |  |  | 0 |
| Crosswalk Width(ft) | 16 |  | 16 |  |  | 16 |
| Two way Left Turn Lane |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 | 9 |  | 9 | 15 |  |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |
| Control Type: Unsignalized |  |  |  |  |  |  |
| Intersection Capacity Utilization 56.7\% |  |  |  | ICU Level of Service |  |  |
| Analysis Period (min) 15 |  |  |  |  |  |  |




|  | 4 | $\rightarrow$ |  | 6 |  | 4 | 4 | 4 | \% | $\pm$ | $\frac{1}{7}$ | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | ${ }^{7}$ | $\uparrow$ |  | ${ }^{1 /}$ | 4 | 「 | ${ }^{7}$ | 4 | 「 |
| Traffic Volume (vph) | 11 | 1 | 5 | 17 | 4 | 79 | 4 | 266 | 26 | 93 | 308 | 10 |
| Future Volume (vph) | 11 | 1 | 5 | 61 | 4 | 254 | 4 | 266 | 68 | 261 | 308 | 10 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 200 |  | 0 | 80 |  | 0 | 180 |  | 230 | 140 |  | 130 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 1 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.871 |  |  | 0.853 |  |  |  | 0.850 |  |  | 0.850 |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1770 | 1622 | 0 | 1770 | 1589 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 1770 | 1622 | 0 | 1770 | 1589 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 626 |  |  | 658 |  |  | 806 |  |  | 578 |  |
| Travel Time (s) |  | 14.2 |  |  | 15.0 |  |  | 15.7 |  |  | 11.3 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.88 | 0.78 | 0.88 | 0.80 | 0.88 | 0.89 | 0.78 |
| Adj. Flow (vph) | 14 | 1 | 6 | 78 | 5 | 289 | 5 | 302 | 85 | 297 | 346 | 13 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 14 | 7 | 0 | 78 | 294 | 0 | 5 | 302 | 85 | 297 | 346 | 13 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |

## Intersection Summary

```
Area Type: Other
```

Control Type: Unsignalized
Intersection Capacity Utilization 37.2\% ICU Level of Service A
Analysis Period (min) 15



[^8]



[^9]|  | 4 | $\rightarrow$ |  |  |  |  | 4 | $\uparrow$ |  | * | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow$ |  |  | * |  | \% | $\hat{\beta}$ |  | ${ }^{7}$ | $\hat{F}$ |  |
| Traffic Volume (vph) | 5 | 48 | 0 | 0 | 49 | 20 | 0 | 0 | 0 | 10 | 0 | 2 |
| Future Volume (vph) | 47 | 48 | 0 | 0 | 49 | 48 | 0 | 0 | 0 | 39 | 0 | 46 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 |  | 0 | 0 |  | 0 | 90 |  | 0 | 90 |  | 0 |
| Storage Lanes | 0 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  |  |  |  | 0.933 |  |  |  |  |  | 0.850 |  |
| Flt Protected |  | 0.976 |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 1818 | 0 | 0 | 1738 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Flt Permitted |  | 0.976 |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 0 | 1818 | 0 | 0 | 1738 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (ft) |  | 716 |  |  | 677 |  |  | 727 |  |  | 714 |  |
| Travel Time (s) |  | 16.3 |  |  | 15.4 |  |  | 16.5 |  |  | 16.2 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 |
| Adj. Flow (vph) | 60 | 62 | 0 | 0 | 63 | 62 | 0 | 0 | 0 | 50 | 0 | 59 |
| Shared Lane Trafic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 122 | 0 | 0 | 125 | 0 | 0 | 0 | 0 | 50 | 59 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 0 |  |  | 0 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(tt) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 60 | 60 |  | 9 | 60 |  | 60 | 15 |  | 9 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 16.7\% ICU Level of Service A
Analysis Period (min) 15

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 4.3 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \& |  |  | \& |  | ${ }^{1}$ | $\uparrow$ |  | ${ }^{1}$ | $\uparrow$ |  |
| Traffic Vol, veh/h | 5 | 48 | 0 | 0 | 49 | 20 | 0 | 0 | 0 | 10 | 0 | 2 |
| Future Vol, veh/h | 47 | 48 | 0 | 0 | 49 | 48 | 0 | 0 | 0 | 39 | 0 | 46 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control Fr | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | 90 | - | - | 90 | - | - |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 78 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 60 | 62 | 0 | 0 | 63 | 62 | 0 | 0 | 0 | 50 | 0 | 59 |



|  | 7 | 4 | 4 | $p$ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | \% | F | $\hat{F}$ |  |  | $\uparrow$ |
| Traffic Volume (vph) | 17 | 71 | 519 | 5 | 32 | 550 |
| Future Volume (vph) | 17 | 71 | 519 | 5 | 32 | 550 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 90 | 0 |  | 0 | 0 |  |
| Storage Lanes | 1 | 1 |  | 0 | 0 |  |
| Taper Length (ft) | 25 |  |  |  | 25 |  |
| Lane Utill. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.850 | 0.999 |  |  |  |
| Flt Protected | 0.950 |  |  |  |  | 0.997 |
| Satd. Flow (prot) | 1770 | 1583 | 1861 | 0 | 0 | 1857 |
| Flt Permitted | 0.950 |  |  |  |  | 0.997 |
| Satd. Flow (perm) | 1770 | 1583 | 1861 | 0 | 0 | 1857 |
| Link Speed (mph) | 30 |  | 35 |  |  | 35 |
| Link Distance (ft) | 725 |  | 578 |  |  | 580 |
| Travel Time (s) | 16.5 |  | 11.3 |  |  | 11.3 |
| Peak Hour Factor | 0.78 | 0.80 | 0.92 | 0.78 | 0.78 | 0.92 |
| Adj. Flow (vph) | 22 | 89 | 564 | 6 | 41 | 598 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |
| Lane Group Flow (vph) | 22 | 89 | 570 | 0 | 0 | 639 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width(ft) | 12 |  | 12 |  |  | 12 |
| Link Offset(ft) | 0 |  | 0 |  |  | 0 |
| Crosswalk Width(ft) | 16 |  | 16 |  |  | 16 |
| Two way Left Turn Lane |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 | 9 |  | 9 | 15 |  |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type: Other |  |  |  |  |  |  |
| Control Type: Unsignalized |  |  |  |  |  |  |
| Intersection Capacity Utilization 65.1\%Analysis Period (min) 15 |  |  |  |  |  |  |
|  |  |  |  |  |  |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | 1 | $\mathbf{7}$ | $\mathbf{F}$ |  |  | $\mathbf{-}$ |
| Traffic Vol, veh/h | 17 | 71 | 519 | 5 | 32 | 550 |
| Future Vol, veh/h | 17 | 71 | 519 | 5 | 32 | 550 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 90 | 0 | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 78 | 80 | 92 | 78 | 78 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 22 | 89 | 564 | 6 | 41 | 598 |



[^10]| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | F |  | \% | F |  | \% | $\uparrow$ | F | ${ }^{7}$ | $\uparrow$ | F |
| Traffic Volume (vph) | 16 | 6 | 8 | 18 |  | 107 | 6 | 434 | 17 | 102 | 436 | 42 |
| Future Volume (vph) | 16 | 6 | 8 | 18 | 2 | 107 | 6 | 434 | 17 | 102 | 436 | 42 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 200 |  | 0 | 80 |  | 0 | 180 |  | 230 | 140 |  | 130 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 1 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.917 |  |  | 0.853 |  |  |  | 0.850 |  |  | 0.850 |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1770 | 1708 | 0 | 1770 | 1589 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.605 |  |  | 0.746 |  |  | 0.482 |  |  | 0.312 |  |  |
| Satd. Flow (perm) | 1127 | 1708 | 0 | 1390 | 1589 | 0 | 898 | 1863 | 1583 | 581 | 1863 | 1583 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 10 |  |  | 129 |  |  |  | 164 |  |  | 164 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 626 |  |  | 658 |  |  | 806 |  |  | 578 |  |
| Travel Time (s) |  | 14.2 |  |  | 15.0 |  |  | 15.7 |  |  | 11.3 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.83 | 0.78 | 0.91 | 0.78 | 0.83 | 0.91 | 0.78 |
| Adj. Flow (vph) | 21 | 8 | 10 | 23 | 3 | 129 | 8 | 477 | 22 | 123 | 479 | 54 |


| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


| Two way Left Turn Lane | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Headway Factor | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |


| Number of Detectors | 1 | 2 | 1 | 2 | 1 | 2 | 1 | 1 | 2 | 1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Detector Template | Left | Thru | Left | Thru | Left | Thru | Right | Left | Thru | Right |
| Leading Detector (ft) | 20 | 100 | 20 | 100 | 20 | 100 | 20 | 20 | 100 | 20 |
| Trailing Detector (ft) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Detector 1 Position(ft) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Detector 1 Size(ft) | 20 | 6 | 20 | 6 | 20 | 6 | 20 | 20 | 6 | 20 |
| Detector 1 Type | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex | Cl+Ex | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex | Cl+Ex |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Detector 1 Queue (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Detector 1 Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Detector 2 Position(ft) |  | 94 |  | 94 |  | 94 |  |  | 94 |  |
| Detector 2 Size(ft) |  | 6 |  | 6 |  | 6 |  |  | 6 |  |
| Detector 2 Type |  | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |


| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Detector 2 Extend (s) |  | 0.0 |  | 0.0 |  | 0.0 |  |  | 0.0 |  |
| Turn Type | pm+pt | NA | pm+pt | NA | pm+pt | NA | Perm | pm+pt | NA | Perm |
| Protected Phases | 7 | 4 | 3 | 8 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  | 8 |  | 2 |  | 2 | 6 |  | 6 |

[^11]|  | 4 |  |  |  |  |  |  | $\uparrow$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Detector Phase | 7 | 4 |  | 3 | 8 |  | 5 | 2 | 2 | 1 | 6 | 6 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 5.0 | 5.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Minimum Split (s) | 10.5 | 23.5 |  | 10.5 | 23.5 |  | 10.5 | 23.5 | 23.5 | 10.5 | 43.5 | 43.5 |
| Total Split (s) | 10.5 | 23.5 |  | 10.5 | 23.5 |  | 10.5 | 40.0 | 40.0 | 16.0 | 45.5 | 45.5 |
| Total Split (\%) | 11.7\% | 26.1\% |  | 11.7\% | 26.1\% |  | 11.7\% | 44.4\% | 44.4\% | 17.8\% | 50.6\% | 50.6\% |
| Maximum Green (s) | 5.0 | 18.0 |  | 5.0 | 18.0 |  | 5.0 | 34.5 | 34.5 | 10.5 | 40.0 | 40.0 |
| Yellow Time (s) | 3.5 | 3.5 |  | 3.5 | 3.5 |  | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 |
| Lead/Lag | Lead | Lag |  | Lead | Lag |  | Lead | Lag | Lag | Lead | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes |  | Yes | Yes |  | Yes | Yes | Yes | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Recall Mode | None | None |  | None | None |  | None | Min | Min | None | Min | Min |
| Walk Time (s) |  | 7.0 |  |  | 7.0 |  |  | 7.0 | 7.0 |  | 7.0 | 7.0 |
| Flash Dont Walk (s) |  | 11.0 |  |  | 11.0 |  |  | 11.0 | 11.0 |  | 11.0 | 11.0 |
| Pedestrian Calls (\#/hr) |  | 0 |  |  | 0 |  |  | 0 | 0 |  | 0 | 0 |
| Act Effct Green (s) | 9.4 | 8.9 |  | 8.5 | 7.2 |  | 29.2 | 27.1 | 27.1 | 35.3 | 35.8 | 35.8 |
| Actuated g/C Ratio | 0.17 | 0.16 |  | 0.16 | 0.13 |  | 0.54 | 0.50 | 0.50 | 0.65 | 0.66 | 0.66 |
| V/c Ratio | 0.08 | 0.06 |  | 0.09 | 0.41 |  | 0.01 | 0.51 | 0.03 | 0.22 | 0.39 | 0.05 |
| Control Delay | 21.7 | 19.3 |  | 21.9 | 11.5 |  | 6.5 | 17.4 | 0.1 | 6.7 | 9.6 | 0.1 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 21.7 | 19.3 |  | 21.9 | 11.5 |  | 6.5 | 17.4 | 0.1 | 6.7 | 9.6 | 0.1 |
| LOS | C | B |  | C | B |  | A | B | A | A | A | A |
| Approach Delay |  | 20.6 |  |  | 13.1 |  |  | 16.4 |  |  | 8.3 |  |
| Approach LOS |  | C |  |  | B |  |  | B |  |  | A |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: | her |  |  |  |  |  |  |  |  |  |  |  |

Cycle Length: 90
Actuated Cycle Length: 54.2
Natural Cycle: 90
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.51
Intersection Signal Delay: $12.2 \quad$ Intersection LOS: B

Intersection Capacity Utilization 49.9\%
ICU Level of Service A
Analysis Period (min) 15
Splits and Phases: 2: Parish Ave \& Settler Way


|  | $\rangle$ | $\rightarrow$ | 7 | 4 | 4 | $\dagger$ | $>$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBL | WBT | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Group Flow (vph) | 21 | 18 | 23 | 132 | 8 | 477 | 22 | 123 | 479 | 54 |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.08 | 0.06 | 0.09 | 0.41 | 0.01 | 0.51 | 0.03 | 0.22 | 0.39 | 0.05 |
| Control Delay | 21.7 | 19.3 | 21.9 | 11.5 | 6.5 | 17.4 | 0.1 | 6.7 | 9.6 | 0.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 21.7 | 19.3 | 21.9 | 11.5 | 6.5 | 17.4 | 0.1 | 6.7 | 9.6 | 0.1 |
| Queue Length 50th ( t ) | 5 | 2 | 6 | 1 | 1 | 102 | 0 | 10 | 48 | 0 |
| Queue Length 95th (ft) | 22 | 19 | 23 | 34 | 6 | 285 | 0 | 42 | 252 | 0 |
| Internal Link Dist (ft) |  | 546 |  | 578 |  | 726 |  |  | 498 |  |
| Turn Bay Length (ft) | 200 |  | 80 |  | 180 |  | 230 | 140 |  | 130 |
| Base Capacity (vph) | 261 | 642 | 257 | 672 | 573 | 1298 | 1153 | 637 | 1446 | 1265 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.08 | 0.03 | 0.09 | 0.20 | 0.01 | 0.37 | 0.02 | 0.19 | 0.33 | 0.04 |

[^12]| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | $\hat{\beta}$ |  | \% | $\hat{\beta}$ |  | \% | $\uparrow$ | 「 | ${ }^{7}$ | $\uparrow$ | F |
| Traffic Volume (veh/h) | 16 | 6 | 8 | 18 |  | 107 | 6 | 434 | 17 | 102 | 436 | 42 |
| Future Volume (veh/h) | 16 | 6 | 8 | 18 | 2 | 107 | 6 | 434 | 17 | 102 | 436 | 42 |
| Initial $Q(Q b)$, veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 21 | 8 | 10 | 23 | 3 | 129 | 8 | 477 | 22 | 123 | 479 | 54 |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.83 | 0.78 | 0.91 | 0.78 | 0.83 | 0.91 | 0.78 |
| Percent Heavy Veh, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 239 | 88 | 110 | 343 | 4 | 184 | 334 | 616 | 522 | 382 | 751 | 636 |
| Arrive On Green | 0.03 | 0.12 | 0.12 | 0.03 | 0.12 | 0.12 | 0.01 | 0.33 | 0.33 | 0.08 | 0.40 | 0.40 |
| Sat Flow, veh/h | 1781 | 756 | 945 | 1781 | 36 | 1554 | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 |
| Grp Volume(v), veh/h | 21 | 0 | 18 | 23 | 0 | 132 | 8 | 477 | 22 | 123 | 479 | 54 |
| Grp Sat Flow(s),veh/h/n | 1781 | 0 | 1700 | 1781 | 0 | 1591 | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 |
| Q Serve(g_s), s | 0.5 | 0.0 | 0.5 | 0.6 | 0.0 | 4.0 | 0.1 | 11.4 | 0.5 | 2.2 | 10.2 | 1.0 |
| Cycle Q Clear(g_c), s | 0.5 | 0.0 | 0.5 | 0.6 | 0.0 | 4.0 | 0.1 | 11.4 | 0.5 | 2.2 | 10.2 | 1.0 |
| Prop In Lane | 1.00 |  | 0.56 | 1.00 |  | 0.98 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Lane Grp Cap(c), veh/h | 239 | 0 | 198 | 343 | 0 | 188 | 334 | 616 | 522 | 382 | 751 | 636 |
| V/C Ratio(X) | 0.09 | 0.00 | 0.09 | 0.07 | 0.00 | 0.70 | 0.02 | 0.77 | 0.04 | 0.32 | 0.64 | 0.08 |
| Avail Cap(c_a), veh/h | 374 | 0 | 618 | 474 | 0 | 578 | 495 | 1303 | 1104 | 613 | 1511 | 1280 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(l) | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 18.6 | 0.0 | 19.5 | 18.4 | 0.0 | 21.0 | 11.3 | 14.9 | 11.3 | 10.7 | 11.9 | 9.2 |
| Incr Delay (d2), s/veh | 0.2 | 0.0 | 0.2 | 0.1 | 0.0 | 4.7 | 0.0 | 2.1 | 0.0 | 0.5 | 0.9 | 0.1 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%ile BackOfQ $(50 \%$ ),veh/ln | 0.2 | 0.0 | 0.2 | 0.2 | 0.0 | 1.6 | 0.1 | 4.3 | 0.1 | 0.7 | 3.5 | 0.3 |
| Unsig. Movement Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay (d),s/veh | 18.8 | 0.0 | 19.7 | 18.5 | 0.0 | 25.7 | 11.3 | 17.1 | 11.3 | 11.2 | 12.8 | 9.2 |
| LnGrp LOS | B | A | B | B | A | C | B | B | B | B | B | A |
| Approach Vol, veh/h |  | 39 |  |  | 155 |  |  | 507 |  |  | 656 |  |
| Approach Delay, s/veh |  | 19.2 |  |  | 24.6 |  |  | 16.7 |  |  | 12.2 |  |
| Approach LOS |  | B |  |  | C |  |  | B |  |  | B |  |


| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Phs Duration $(G+Y+R c)$ s | 9.6 | 21.8 | 6.9 | 11.3 | 6.0 | 25.4 | 6.8 | 11.4 |
| Change Period (Y+Rc), s | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 |
| Max Green Setting (Gmax), s | 10.5 | 34.5 | 5.0 | 18.0 | 5.0 | 40.0 | 5.0 | 18.0 |
| Max Q Clear Time (g_c+11), s | 4.2 | 13.4 | 2.6 | 2.5 | 2.1 | 12.2 | 2.5 | 6.0 |
| Green Ext Time (p_c), s | 0.1 | 3.0 | 0.0 | 0.0 | 0.0 | 3.3 | 0.0 | 0.5 |

Intersection Summary
HCM 6th Ctrl Delay 15.5
HCM 6th LOS B


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh | 6.8 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \＆ |  | ${ }^{7}$ | 4 | 「 | ${ }^{7}$ | 4 | 「゙ | ${ }^{1 /}$ | 4 | 「 |
| Traffic Vol，veh／h | 52 | 10 | 44 | 40 | 10 | 70 | 10 | 329 | 29 | 60 | 354 | 28 |
| Future Vol，veh／h | 52 | 10 | 44 | 40 | 10 | 70 | 10 | 329 | 29 | 60 | 354 | 28 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | － | － | None | － | － | None | － | － | None | － | － | None |
| Storage Length | － | － | － | 150 | － | 150 | 240 | － | 290 | 260 | － | 200 |
| Veh in Median Storage，\＃ | \＃ | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Grade，\％ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 80 | 78 | 89 | 78 | 79 | 90 | 78 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 67 | 13 | 56 | 51 | 13 | 88 | 13 | 370 | 37 | 76 | 393 | 36 |



[^13]|  | 4 | $\rightarrow$ |  | 7 |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 1 |  |  | ${ }_{\text {¢ }}$ |  | ${ }^{7}$ | $\hat{F}$ |  | ${ }^{7}$ | $\hat{F}$ |  |
| Traffic Volume (vph) | 1 | 98 | 0 | 0 | 116 | 10 | 0 | 0 | 0 | 14 | 0 | 4 |
| Future Volume (vph) | 1 | 98 | 0 | 0 | 116 | 10 | 0 | 0 | 0 | 14 | 0 | 4 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 150 |  | 0 | 0 |  | 0 | 90 |  | 0 | 90 |  | 0 |
| Storage Lanes | 1 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  |  |  |  | 0.988 |  |  |  |  |  | 0.850 |  |
| Flt Protected | 0.950 |  |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1770 | 1863 | 0 | 0 | 1840 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Flt Permitted | 0.950 |  |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 1770 | 1863 | 0 | 0 | 1840 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (ft) |  | 716 |  |  | 677 |  |  | 727 |  |  | 714 |  |
| Travel Time (s) |  | 16.3 |  |  | 15.4 |  |  | 16.5 |  |  | 16.2 |  |
| Peak Hour Factor | 0.78 | 0.83 | 0.78 | 0.78 | 0.84 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 |
| Adj. Flow (vph) | 1 | 118 | 0 | 0 | 138 | 13 | 0 | 0 | 0 | 18 | 0 | 5 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 1 | 118 | 0 | 0 | 151 | 0 | 0 | 0 | 0 | 18 | 5 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 60 | 60 |  | 9 | 60 |  | 60 | 15 |  | 9 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 16.7\% ICU Level of Service A
Analysis Period (min) 15



[^14]



[^15]| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | $\uparrow$ |  | \% | $\uparrow$ |  | \% | 4 | 「 | \% | 4 | 「 |
| Traffic Volume (vph) | 20 | 2 | 10 | 30 | 8 | 143 | 8 | 485 | 46 | 164 | 570 | 18 |
| Future Volume (vph) | 20 | 2 | 10 | 30 | 8 | 143 | 8 | 485 | 46 | 164 | 570 | 18 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 200 |  | 0 | 80 |  | 0 | 180 |  | 230 | 140 |  | 130 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 1 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.878 |  |  | 0.858 |  |  |  | 0.850 |  |  | 0.850 |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1770 | 1635 | 0 | 1770 | 1598 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.656 |  |  | 0.549 |  |  | 0.377 |  |  | 0.235 |  |  |
| Satd. Flow (perm) | 1222 | 1635 | 0 | 1023 | 1598 | 0 | 702 | 1863 | 1583 | 438 | 1863 | 1583 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 13 |  |  | 168 |  |  |  | 230 |  |  | 164 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 626 |  |  | 658 |  |  | 806 |  |  | 578 |  |
| Travel Time (s) |  | 14.2 |  |  | 15.0 |  |  | 15.7 |  |  | 11.3 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.85 | 0.78 | 0.92 | 0.78 | 0.86 | 0.92 | 0.78 |
| Adj. Flow (vph) | 26 | 3 | 13 | 38 | 10 | 168 | 10 | 527 | 59 | 191 | 620 | 23 |


| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Lane Group Flow (vph) | 26 | 16 | 0 | 38 | 178 | 0 | 10 | 527 | 59 | 191 | 620 | 23 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |


| Two way Left Turn Lane | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Headway Factor | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |


| Number of Detectors | 1 | 2 | 1 | 2 | 1 | 2 | 1 | 1 | 2 | 1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Detector Template | Left | Thru | Left | Thru | Left | Thru | Right | Left | Thru | Right |
| Leading Detector (ft) | 20 | 100 | 20 | 100 | 20 | 100 | 20 | 20 | 100 | 20 |
| Trailing Detector (ft) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Detector 1 Position(ft) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Detector 1 Size(ft) | 20 | 6 | 20 | 6 | 20 | 6 | 20 | 20 | 6 | 20 |
| Detector 1 Type | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Detector 1 Queue (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Detector 1 Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Detector 2 Position(ft) |  | 94 |  | 94 |  | 94 |  |  | 94 |  |
| Detector 2 Size(ft) |  | 6 |  | 6 |  | 6 |  |  | 6 |  |
| Detector 2 Type |  | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |


| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Detector 2 Extend (s) |  | 0.0 |  | 0.0 |  | 0.0 |  |  | 0.0 |  |
| Turn Type | pm+pt | NA | pm+pt | NA | pm+pt | NA | Perm | pm+pt | NA | Perm |
| Protected Phases | 7 | 4 | 3 | 8 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  | 8 |  | 2 |  | 2 | 6 |  |  |

[^16]|  | 4 |  |  |  |  |  | 4 | 4 |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Detector Phase | 7 | 4 |  | 3 | 8 |  | 5 | 2 | 2 | 1 | 6 | 6 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 5.0 | 5.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Minimum Split (s) | 10.5 | 23.5 |  | 10.5 | 23.5 |  | 10.5 | 23.5 | 23.5 | 10.5 | 23.5 | 23.5 |
| Total Split (s) | 10.5 | 23.5 |  | 10.5 | 23.5 |  | 10.5 | 35.0 | 35.0 | 21.0 | 45.5 | 45.5 |
| Total Split (\%) | 11.7\% | 26.1\% |  | 11.7\% | 26.1\% |  | 11.7\% | 38.9\% | 38.9\% | 23.3\% | 50.6\% | 50.6\% |
| Maximum Green (s) | 5.0 | 18.0 |  | 5.0 | 18.0 |  | 5.0 | 29.5 | 29.5 | 15.5 | 40.0 | 40.0 |
| Yellow Time (s) | 3.5 | 3.5 |  | 3.5 | 3.5 |  | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 |
| Lead/Lag | Lead | Lag |  | Lead | Lag |  | Lead | Lag | Lag | Lead | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes |  | Yes | Yes |  | Yes | Yes | Yes | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Recall Mode | None | None |  | None | None |  | None | Min | Min | None | Min | Min |
| Walk Time (s) |  | 7.0 |  |  | 7.0 |  |  | 7.0 | 7.0 |  | 7.0 | 7.0 |
| Flash Dont Walk (s) |  | 11.0 |  |  | 11.0 |  |  | 11.0 | 11.0 |  | 11.0 | 11.0 |
| Pedestrian Calls (\#/hr) |  | 0 |  |  | 0 |  |  | 0 | 0 |  | 0 | 0 |
| Act Effct Green (s) | 7.6 | 7.1 |  | 9.8 | 7.4 |  | 28.5 | 23.1 | 23.1 | 37.8 | 36.3 | 36.3 |
| Actuated g/C Ratio | 0.13 | 0.12 |  | 0.16 | 0.12 |  | 0.47 | 0.38 | 0.38 | 0.63 | 0.60 | 0.60 |
| v/c Ratio | 0.13 | 0.08 |  | 0.15 | 0.52 |  | 0.02 | 0.74 | 0.08 | 0.41 | 0.55 | 0.02 |
| Control Delay | 27.4 | 19.3 |  | 23.6 | 12.9 |  | 7.1 | 25.0 | 0.2 | 8.4 | 11.9 | 0.1 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 27.4 | 19.3 |  | 23.6 | 12.9 |  | 7.1 | 25.0 | 0.2 | 8.4 | 11.9 | 0.1 |
| LOS | C | B |  | C | B |  | A | C | A | A | B | A |
| Approach Delay |  | 24.3 |  |  | 14.8 |  |  | 22.2 |  |  | 10.8 |  |
| Approach LOS |  | C |  |  | B |  |  | C |  |  | B |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: | her |  |  |  |  |  |  |  |  |  |  |  |

Cycle Length: 90
Actuated Cycle Length: 60.2
Natural Cycle: 80
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.74

```
Intersection Signal Delay: 15.7 Intersection LOS: B
```

Intersection Capacity Utilization 65.0\% ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: 2: Parish Ave \& Settler Way


[^17]|  | $\rangle$ | $\rightarrow$ | 7 |  | 4 | $\dagger$ | $p$ | - | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBL | WBT | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Group Flow (vph) | 26 | 16 | 38 | 178 | 10 | 527 | 59 | 191 | 620 | 23 |
| v/c Ratio | 0.13 | 0.08 | 0.15 | 0.52 | 0.02 | 0.74 | 0.08 | 0.41 | 0.55 | 0.02 |
| Control Delay | 27.4 | 19.3 | 23.6 | 12.9 | 7.1 | 25.0 | 0.2 | 8.4 | 11.9 | 0.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 27.4 | 19.3 | 23.6 | 12.9 | 7.1 | 25.0 | 0.2 | 8.4 | 11.9 | 0.1 |
| Queue Length 50th (ft) | 7 | 1 | 11 | 3 | 1 | 122 | 0 | 16 | 71 | 0 |
| Queue Length 95th (ft) | 25 | 15 | 33 | 40 | 7 | \#370 | 0 | 67 | 372 | 0 |
| Internal Link Dist (ft) |  | 546 |  | 578 |  | 726 |  |  | 498 |  |
| Turn Bay Length (ft) | 200 |  | 80 |  | 180 |  | 230 | 140 |  | 130 |
| Base Capacity (vph) | 202 | 532 | 260 | 626 | 427 | 985 | 945 | 642 | 1351 | 1193 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.13 | 0.03 | 0.15 | 0.28 | 0.02 | 0.54 | 0.06 | 0.30 | 0.46 | 0.02 |

Intersection Summary
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | $\hat{F}$ |  | ${ }^{7}$ | $\hat{\beta}$ |  | \% | $\uparrow$ | F' | ${ }^{7}$ | $\uparrow$ | F |
| Traffic Volume (veh/h) | 20 |  | 10 | 30 | 8 | 143 | 8 | 485 | 46 | 164 | 570 | 18 |
| Future Volume (veh/h) | 20 | 2 | 10 | 30 | 8 | 143 | 8 | 485 | 46 | 164 | 570 | 18 |
| Initial $Q(Q b)$, veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 26 | 3 | 13 | 38 | 10 | 168 | 10 | 527 | 59 | 191 | 620 | 23 |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.85 | 0.78 | 0.92 | 0.78 | 0.86 | 0.92 | 0.78 |
| Percent Heavy Veh, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 228 | 43 | 184 | 377 | 13 | 225 | 261 | 639 | 541 | 371 | 801 | 679 |
| Arrive On Green | 0.03 | 0.14 | 0.14 | 0.04 | 0.15 | 0.15 | 0.01 | 0.34 | 0.34 | 0.10 | 0.43 | 0.43 |
| Sat Flow, veh/h | 1781 | 306 | 1326 | 1781 | 90 | 1509 | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 |
| Grp Volume(v), veh/h | 26 | 0 | 16 | 38 | 0 | 178 | 10 | 527 | 59 | 191 | 620 | 23 |
| Grp Sat Flow(s),veh/h/n | 1781 | 0 | 1632 | 1781 | 0 | 1599 | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 |
| Q Serve(g_s), s | 0.7 | 0.0 | 0.5 | 1.0 | 0.0 | 6.2 | 0.2 | 14.9 | 1.5 | 3.7 | 16.4 | 0.5 |
| Cycle Q Clear(g_c), s | 0.7 | 0.0 | 0.5 | 1.0 | 0.0 | 6.2 | 0.2 | 14.9 | 1.5 | 3.7 | 16.4 | 0.5 |
| Prop In Lane | 1.00 |  | 0.81 | 1.00 |  | 0.94 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Lane Grp Cap(c), veh/h | 228 | 0 | 227 | 377 | 0 | 238 | 261 | 639 | 541 | 371 | 801 | 679 |
| V/C Ratio(X) | 0.11 | 0.00 | 0.07 | 0.10 | 0.00 | 0.75 | 0.04 | 0.82 | 0.11 | 0.51 | 0.77 | 0.03 |
| Avail Cap(c_a), veh/h | 330 | 0 | 508 | 461 | 0 | 497 | 392 | 953 | 808 | 671 | 1293 | 1096 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(l) | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 20.6 | 0.0 | 21.7 | 20.0 | 0.0 | 23.6 | 13.3 | 17.5 | 13.0 | 12.2 | 14.1 | 9.6 |
| Incr Delay (d2), s/veh | 0.2 | 0.0 | 0.1 | 0.1 | 0.0 | 4.6 | 0.1 | 3.8 | 0.1 | 1.1 | 1.6 | 0.0 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%ile BackOfQ(50\%),veh/ln | 0.3 | 0.0 | 0.2 | 0.4 | 0.0 | 2.5 | 0.1 | 6.2 | 0.5 | 1.3 | 6.0 | 0.2 |
| Unsig. Movement Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay(d),s/veh | 20.8 | 0.0 | 21.8 | 20.1 | 0.0 | 28.2 | 13.3 | 21.2 | 13.1 | 13.3 | 15.8 | 9.6 |
| LnGrp LOS | C | A | C | C | A | C | B | C | B | B | B | A |
| Approach Vol, veh/h |  | 42 |  |  | 216 |  |  | 596 |  |  | 834 |  |
| Approach Delay, s/veh |  | 21.2 |  |  | 26.8 |  |  | 20.3 |  |  | 15.0 |  |
| Approach LOS |  | C |  |  | C |  |  | C |  |  | B |  |


| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Phs Duration $(G+Y+R c)$, s | 11.3 | 25.3 | 7.8 | 13.5 | 6.2 | 30.3 | 7.2 | 14.1 |
| Change Period (Y+Rc), s | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 |
| Max Green Setting (Gmax), s | 15.5 | 29.5 | 5.0 | 18.0 | 5.0 | 40.0 | 5.0 | 18.0 |
| Max Q Clear Time (g_c+11), s | 5.7 | 16.9 | 3.0 | 2.5 | 2.2 | 18.4 | 2.7 | 8.2 |
| Green Ext Time (p_c), s | 0.3 | 2.8 | 0.0 | 0.0 | 0.0 | 4.2 | 0.0 | 0.7 |

Intersection Summary

| HCM 6th Ctrl Delay | 18.6 |
| :--- | ---: |
| HCM 6th LOS | $B$ |

[^18]



[^19]|  | $\stackrel{ }{*}$ |  |  | 7 |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\hat{F}$ |  |  | ¢ |  | ${ }^{7}$ | $\hat{1}$ |  | ${ }^{7}$ | 1 |  |
| Traffic Volume (vph) | 6 | 90 | 0 | 0 | 92 | 29 | 0 | 0 | 0 | 15 | 0 | 2 |
| Future Volume (vph) | 6 | 90 | 0 | 0 | 92 | 29 | 0 | 0 | 0 | 15 | 0 | 2 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 150 |  | 0 | 0 |  | 0 | 90 |  | 0 | 90 |  | 0 |
| Storage Lanes | 1 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Utill. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  |  |  |  | 0.966 |  |  |  |  |  | 0.850 |  |
| Flt Protected | 0.950 |  |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1770 | 1863 | 0 | 0 | 1799 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Flt Permitted | 0.950 |  |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 1770 | 1863 | 0 | 0 | 1799 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (ft) |  | 716 |  |  | 677 |  |  | 727 |  |  | 714 |  |
| Travel Time (s) |  | 16.3 |  |  | 15.4 |  |  | 16.5 |  |  | 16.2 |  |
| Peak Hour Factor | 0.78 | 0.82 | 0.78 | 0.78 | 0.82 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 |
| Adj. Flow (vph) | 8 | 110 | 0 | 0 | 112 | 37 | 0 | 0 | 0 | 19 | 0 | 3 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 8 | 110 | 0 | 0 | 149 | 0 | 0 | 0 | 0 | 19 | 3 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 60 | 60 |  | 9 | 60 |  | 60 | 15 |  |  |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 16.6\% ICU Level of Service A

Analysis Period (min) 15



[^20]



| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ | 「 | \% | $\uparrow$ | F |
| Trafic Volume (vph) | 16 | 6 | 8 | 18 | 2 | 107 | 6 | 434 | 17 | 102 | 436 | 42 |
| Future Volume (vph) | 16 | 6 | 8 | 29 | 2 | 150 | 6 | 434 | 35 | 173 | 436 | 42 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 200 |  | 0 | 80 |  | 0 | 180 |  | 230 | 140 |  | 130 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 1 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.917 |  |  | 0.853 |  |  |  | 0.850 |  |  | 0.850 |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1770 | 1708 | 0 | 1770 | 1589 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.667 |  |  | 0.552 |  |  | 0.490 |  |  | 0.263 |  |  |
| Satd. Flow (perm) | 1242 | 1708 | 0 | 1028 | 1589 | 0 | 913 | 1863 | 1583 | 490 | 1863 | 1583 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 10 |  |  | 176 |  |  |  | 164 |  |  | 164 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance ( ft ) |  | 626 |  |  | 658 |  |  | 806 |  |  | 578 |  |
| Travel Time (s) |  | 14.2 |  |  | 15.0 |  |  | 15.7 |  |  | 11.3 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.85 | 0.78 | 0.91 | 0.78 | 0.86 | 0.91 | 0.78 |
| Adj. Flow (vph) | 21 | 8 | 10 | 37 | 3 | 176 |  | 477 | 45 | 201 | 479 | 54 |


| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |


| Two way Left Turn Lane | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Headway Factor | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |


| Number of Detectors | 1 | 2 | 1 | 2 | 1 | 2 | 1 | 1 | 2 | 1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Detector Template | Left | Thru | Left | Thru | Left | Thru | Right | Left | Thru | Right |
| Leading Detector (ft) | 20 | 100 | 20 | 100 | 20 | 100 | 20 | 20 | 100 | 20 |
| Trailing Detector (ft) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Detector 1 Position(ft) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Detector 1 Size(ft) | 20 | 6 | 20 | 6 | 20 | 6 | 20 | 20 | 6 | 20 |
| Detector 1 Type | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex | Cl+Ex | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex | Cl+Ex |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Detector 1 Queue (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Detector 1 Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Detector 2 Position(ft) |  | 94 |  | 94 |  | 94 |  |  | 94 |  |
| Detector 2 Size(ft) |  | 6 |  | 6 |  | 6 |  |  | 6 |  |
| Detector 2 Type |  | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl+Ex |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |


| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Detector 2 Extend (s) |  | 0.0 |  | 0.0 |  | 0.0 |  |  | 0.0 |  |
| Turn Type | pm+pt | NA | pm+pt | NA | pm+pt | NA | Perm | pm+pt | NA | Perm |
| Protected Phases | 7 | 4 | 3 | 8 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  | 8 |  | 2 |  | 2 | 6 |  |  |

[^21]|  | $\stackrel{ }{*}$ |  |  |  |  |  | 4 | 4 |  | $\checkmark$ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Detector Phase | 7 | 4 |  | 3 | 8 |  | 5 | 2 | 2 | 1 | 6 | 6 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 5.0 | 5.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Minimum Split (s) | 10.5 | 23.5 |  | 10.5 | 23.5 |  | 10.5 | 23.5 | 23.5 | 10.5 | 23.5 | 23.5 |
| Total Split (s) | 10.5 | 23.5 |  | 10.5 | 23.5 |  | 10.5 | 40.0 | 40.0 | 16.0 | 45.5 | 45.5 |
| Total Split (\%) | 11.7\% | 26.1\% |  | 11.7\% | 26.1\% |  | 11.7\% | 44.4\% | 44.4\% | 17.8\% | 50.6\% | 50.6\% |
| Maximum Green (s) | 5.0 | 18.0 |  | 5.0 | 18.0 |  | 5.0 | 34.5 | 34.5 | 10.5 | 40.0 | 40.0 |
| Yellow Time (s) | 3.5 | 3.5 |  | 3.5 | 3.5 |  | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 |
| Lead/Lag | Lead | Lag |  | Lead | Lag |  | Lead | Lag | Lag | Lead | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes |  | Yes | Yes |  | Yes | Yes | Yes | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Recall Mode | None | None |  | None | None |  | None | Min | Min | None | Min | Min |
| Walk Time (s) |  | 7.0 |  |  | 7.0 |  |  | 7.0 | 7.0 |  | 7.0 | 7.0 |
| Flash Dont Walk (s) |  | 11.0 |  |  | 11.0 |  |  | 11.0 | 11.0 |  | 11.0 | 11.0 |
| Pedestrian Calls (\#/hr) |  | 0 |  |  | 0 |  |  | 0 | 0 |  | 0 | 0 |
| Act Effct Green (s) | 7.5 | 7.0 |  | 9.6 | 7.3 |  | 26.1 | 20.6 | 20.6 | 35.2 | 33.7 | 33.7 |
| Actuated g/C Ratio | 0.13 | 0.12 |  | 0.17 | 0.13 |  | 0.45 | 0.36 | 0.36 | 0.61 | 0.59 | 0.59 |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.10 | 0.08 |  | 0.14 | 0.51 |  | 0.02 | 0.71 | 0.07 | 0.41 | 0.44 | 0.05 |
| Control Delay | 26.3 | 22.2 |  | 22.7 | 11.7 |  | 6.8 | 23.5 | 0.2 | 8.5 | 10.3 | 0.1 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 26.3 | 22.2 |  | 22.7 | 11.7 |  | 6.8 | 23.5 | 0.2 | 8.5 | 10.3 | 0.1 |
| LOS | C | C |  | C | B |  | A | C | A | A | B | A |
| Approach Delay |  | 24.4 |  |  | 13.6 |  |  | 21.3 |  |  | 9.1 |  |
| Approach LOS |  | C |  |  | B |  |  | C |  |  | A |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: | her |  |  |  |  |  |  |  |  |  |  |  |

Cycle Length: 90
Actuated Cycle Length: 57.5
Natural Cycle: 75
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.71
Intersection Signal Delay: 14.4 Intersection LOS: B

Intersection Capacity Utilization 49.9\%
ICU Level of Service A
Analysis Period (min) 15
Splits and Phases: 2: Parish Ave \& Settler Way


2: Parish Ave \& Settler Way

|  | 4 | $\rightarrow$ | 7 | 4 | 4 | 4 | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBL | WBT | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Group Flow (vph) | 21 | 18 | 37 | 179 | 8 | 477 | 45 | 201 | 479 | 54 |
| v/c Ratio | 0.10 | 0.08 | 0.14 | 0.51 | 0.02 | 0.71 | 0.07 | 0.41 | 0.44 | 0.05 |
| Control Delay | 26.3 | 22.2 | 22.7 | 11.7 | 6.8 | 23.5 | 0.2 | 8.5 | 10.3 | 0.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 26.3 | 22.2 | 22.7 | 11.7 | 6.8 | 23.5 | 0.2 | 8.5 | 10.3 | 0.1 |
| Queue Length 50th ( t ) | 5 | 2 | 10 | 1 | 1 | 106 | 0 | 17 | 48 | 0 |
| Queue Length 95th (ft) | 22 | 19 | 32 | 37 | 6 | 294 | 0 | 70 | 259 | 0 |
| Internal Link Dist (tt) |  | 546 |  | 578 |  | 726 |  |  | 498 |  |
| Turn Bay Length (ft) | 200 |  | 80 |  | 180 |  | 230 | 140 |  | 130 |
| Base Capacity (vph) | 211 | 587 | 270 | 656 | 494 | 1214 | 1088 | 553 | 1419 | 1245 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.10 | 0.03 | 0.14 | 0.27 | 0.02 | 0.39 | 0.04 | 0.36 | 0.34 | 0.04 |

[^22]| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | $\uparrow$ |  | \% | $\hat{F}$ |  | \% | $\uparrow$ | 「 | \% | $\uparrow$ | F |
| Traffic Volume (veh/h) | 16 | 6 | 8 | 18 | 2 | 107 | 6 | 434 | 17 | 102 | 436 | 42 |
| Future Volume (veh/h) | 16 | 6 | 8 | 29 | 2 | 150 | 6 | 434 | 35 | 173 | 436 | 42 |
| Initial $\mathrm{Q}(\mathrm{Qb})$, veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 21 | 8 | 10 | 37 | 3 | 176 | 8 | 477 | 45 | 201 | 479 | 54 |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.85 | 0.78 | 0.91 | 0.78 | 0.86 | 0.91 | 0.78 |
| Percent Heavy Veh, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 228 | 104 | 130 | 379 | 4 | 237 | 335 | 602 | 510 | 395 | 777 | 659 |
| Arrive On Green | 0.02 | 0.14 | 0.14 | 0.04 | 0.15 | 0.15 | 0.01 | 0.32 | 0.32 | 0.10 | 0.42 | 0.42 |
| Sat Flow, veh/h | 1781 | 756 | 945 | 1781 | 27 | 1562 | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 |
| Grp Volume(v), veh/h | 21 | 0 | 18 | 37 | 0 | 179 | 8 | 477 | 45 | 201 | 479 | 54 |
| Grp Sat Flow(s),veh/h/ln | 1781 | 0 | 1700 | 1781 | 0 | 1589 | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 |
| Q Serve(g_s), s | 0.6 | 0.0 | 0.5 | 1.0 | 0.0 | 6.0 | 0.2 | 12.9 | 1.1 | 3.8 | 11.1 | 1.1 |
| Cycle Q Clear(g_c), s | 0.6 | 0.0 | 0.5 | 1.0 | 0.0 | 6.0 | 0.2 | 12.9 | 1.1 | 3.8 | 11.1 | 1.1 |
| Prop In Lane | 1.00 |  | 0.56 | 1.00 |  | 0.98 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Lane Grp Cap (c), veh/h | 228 | 0 | 234 | 379 | 0 | 241 | 335 | 602 | 510 | 395 | 777 | 659 |
| V/C Ratio(X) | 0.09 | 0.00 | 0.08 | 0.10 | 0.00 | 0.74 | 0.02 | 0.79 | 0.09 | 0.51 | 0.62 | 0.08 |
| Avail Cap(c_a), veh/h | 344 | 0 | 552 | 470 | 0 | 516 | 478 | 1165 | 987 | 546 | 1350 | 1144 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(l) | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 19.9 | 0.0 | 20.8 | 19.2 | 0.0 | 22.5 | 12.7 | 17.1 | 13.1 | 11.7 | 12.7 | 9.8 |
| Incr Delay (d2), s/veh | 0.2 | 0.0 | 0.1 | 0.1 | 0.0 | 4.5 | 0.0 | 2.4 | 0.1 | 1.0 | 0.8 | 0.1 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%oile BackOfQ(50\%),veh/ln | 0.2 | 0.0 | 0.2 | 0.4 | 0.0 | 2.3 | 0.1 | 5.1 | 0.4 | 1.3 | 4.0 | 0.3 |
| Unsig. Movement Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay(d),s/veh | 20.1 | 0.0 | 21.0 | 19.3 | 0.0 | 26.9 | 12.8 | 19.5 | 13.2 | 12.7 | 13.5 | 9.8 |
| LnGrp LOS | C | A | C | B | A | C | B | B | B | B | B | A |
| Approach Vol, veh/h |  | 39 |  |  | 216 |  |  | 530 |  |  | 734 |  |
| Approach Delay, s/veh |  | 20.5 |  |  | 25.6 |  |  | 18.9 |  |  | 13.0 |  |
| Approach LOS |  | C |  |  | C |  |  | B |  |  | B |  |


| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Phs Duration $(G+Y+R c)$, s | 11.3 | 23.3 | 7.7 | 13.1 | 6.1 | 28.5 | 6.9 | 13.9 |
| Change Period (Y+Rc), s | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 |
| Max Green Setting (Gmax), s | 10.5 | 34.5 | 5.0 | 18.0 | 5.0 | 40.0 | 5.0 | 18.0 |
| Max Q Clear Time (g_c+11), s | 5.8 | 14.9 | 3.0 | 2.5 | 2.2 | 13.1 | 2.6 | 8.0 |
| Green Ext Time (p_c), s | 0.2 | 3.0 | 0.0 | 0.0 | 0.0 | 3.3 | 0.0 | 0.7 |

Intersection Summary
HCM 6th Ctrl Delay 17.0

HCM 6th LOS B

|  | $\rangle$ | $\rightarrow$ |  | 7 | $\leftarrow$ | 4 | 4 | $\uparrow$ | + |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  | \% | $\uparrow$ | 7 | \% | 4 | 「 | ${ }^{7}$ | 4 | F |
| Traffic Volume (vph) | 52 | 10 | 44 | 40 | 10 | 70 | 10 | 329 | 29 | 60 | 354 | 28 |
| Future Volume (vph) | 52 | 10 | 44 | 51 | 10 | 70 | 10 | 347 | 47 | 60 | 365 | 28 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 0 |  | 0 | 150 |  | 150 | 240 |  | 290 | 260 |  | 200 |
| Storage Lanes | 0 |  | 0 | 1 |  | 1 | 1 |  | 1 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.944 |  |  |  | 0.850 |  |  | 0.850 |  |  | 0.850 |
| Flt Protected |  | 0.976 |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 0 | 1716 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Flt Permitted |  | 0.976 |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 0 | 1716 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 516 |  |  | 716 |  |  | 680 |  |  | 806 |  |
| Travel Time (s) |  | 11.7 |  |  | 16.3 |  |  | 13.2 |  |  | 15.7 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.80 | 0.78 | 0.89 | 0.78 | 0.79 | 0.90 | 0.78 |
| Adj. Flow (vph) | 67 | 13 | 56 | 65 | 13 | 88 | 13 | 390 | 60 | 76 | 406 | 36 |
| Shared Lane Trafic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 136 | 0 | 65 | 13 | 88 | 13 | 390 | 60 | 76 | 406 | 36 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(f) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | , |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Sign Control |  | Stop |  |  | Stop |  |  | Free |  |  | Free |  |

## Intersection Summary

```
Area Type: Other
```

Control Type: Unsignalized
Intersection Capacity Utilization 44.7\% ICU Level of Service A
Analysis Period (min) 15

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh | 7.5 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \＆ |  | ${ }^{7}$ | 4 | 「 | ${ }^{7}$ | 4 | 「゙ | ${ }^{1 /}$ | 4 | 「 |
| Traffic Vol，veh／h | 52 | 10 | 44 | 40 | 10 | 70 | 10 | 329 | 29 | 60 | 354 | 28 |
| Future Vol，veh／h | 52 | 10 | 44 | 51 | 10 | 70 | 10 | 347 | 47 | 60 | 365 | 28 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | － | － | None | － | － | None | － | － | None | － | － | None |
| Storage Length | － | － | － | 150 | － | 150 | 240 | － | 290 | 260 | － | 200 |
| Veh in Median Storage，\＃ | \＃ | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Grade，\％ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 80 | 78 | 89 | 78 | 79 | 90 | 78 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 67 | 13 | 56 | 65 | 13 | 88 | 13 | 390 | 60 | 76 | 406 | 36 |



| Minor Lane／Major Mvmt | NBL | NBT | NBR EBLn1WBLn1WBLn2WBLn3 | SBL | SBT | SBR |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity（veh／h） | 1118 | - | - | 240 | 174 | 221 | 658 | 1110 | - |


|  | 4 | $\rightarrow$ |  | 7 |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\hat{F}$ |  |  | $\uparrow$ |  | 7 | F |  | \% | $\hat{F}$ |  |
| Traffic Volume (vph) | 1 | 98 | 0 | 0 | 116 | 10 | 0 | 0 | 0 | 14 | 0 | 4 |
| Future Volume (vph) | 19 | 98 | 0 | 0 | 116 | 22 | 0 | 0 | 0 | 21 | 0 | 15 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 150 |  | 0 | 0 |  | 0 | 90 |  | 0 | 90 |  | 0 |
| Storage Lanes | 1 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  |  |  |  | 0.977 |  |  |  |  |  | 0.850 |  |
| Flt Protected | 0.950 |  |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1770 | 1863 | 0 | 0 | 1820 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Flt Permitted | 0.950 |  |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 1770 | 1863 | 0 | 0 | 1820 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (ft) |  | 716 |  |  | 677 |  |  | 727 |  |  | 714 |  |
| Travel Time (s) |  | 16.3 |  |  | 15.4 |  |  | 16.5 |  |  | 16.2 |  |
| Peak Hour Factor | 0.78 | 0.83 | 0.78 | 0.78 | 0.84 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 |
| Adj. Flow (vph) | 24 | 118 | 0 | 0 | 138 | 28 | 0 | 0 | 0 | 27 | 0 | 19 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 24 | 118 | 0 | 0 | 166 | 0 | 0 | 0 | 0 | 27 | 19 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(tt) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 60 | 60 |  | 9 | 60 |  | 60 | 15 |  | 9 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 16.7\% ICU Level of Service A
Analysis Period (min) 15

| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 1.8 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{1}$ | F |  |  | \& |  | ${ }_{1}$ | $\hat{\dagger}$ |  | ${ }^{1}$ | $\uparrow$ |  |
| Traffic Vol, veh/h | 1 | 98 | 0 | 0 | 116 | 10 | 0 | 0 | 0 | 14 | 0 | 4 |
| Future Vol, veh/h | 19 | 98 | 0 | 0 | 116 | 22 | 0 | 0 | 0 | 21 | 0 | 15 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | 150 | - | - | - | - | - | 90 | - | - | 90 | - | - |
| Veh in Median Storage, \# - |  | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 78 | 83 | 78 | 78 | 84 | 78 | 78 | 78 | 78 | 78 | 78 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mumt Flow | 24 | 118 | 0 | 0 | 138 | 28 | 0 | 0 | 0 | 27 | 0 | 19 |






| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | ${ }^{7}$ | $\uparrow$ |  | \% | $\uparrow$ |  | \% | $\uparrow$ | F | \% | $\uparrow$ | F |
| Trafic Volume (vph) | 20 | 2 | 10 | 30 | 8 | 143 | 8 | 485 | 46 | 164 | 570 | 18 |
| Future Volume (vph) | 20 | 2 | 10 | 74 | 8 | 318 | 8 | 485 | 88 | 332 | 570 | 18 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 200 |  | 0 | 80 |  | 0 | 180 |  | 230 | 140 |  | 130 |
| Storage Lanes | 1 |  | 0 | 1 |  | 0 | 1 |  | 1 | 1 |  | 1 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Util. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.878 |  |  | 0.854 |  |  |  | 0.850 |  |  | 0.850 |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1770 | 1635 | 0 | 1770 | 1591 | 0 | 1770 | 1863 | 1583 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.952 |  |  | 0.426 |  |  | 0.430 |  |  | 0.201 |  |  |
| Satd. Flow (perm) | 1773 | 1635 | 0 | 794 | 1591 | 0 | 801 | 1863 | 1583 | 374 | 1863 | 1583 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 13 |  |  | 357 |  |  |  | 230 |  |  | 164 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 35 |  |  | 35 |  |
| Link Distance (ft) |  | 626 |  |  | 658 |  |  | 806 |  |  | 578 |  |
| Travel Time (s) |  | 14.2 |  |  | 15.0 |  |  | 15.7 |  |  | 11.3 |  |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.80 | 0.78 | 0.89 | 0.78 | 0.92 | 0.82 | 0.89 | 0.92 | 0.78 |
| Adj. Flow (vph) | 26 | 3 | 13 | 93 | 10 | 357 | 10 | 527 | 107 | 373 | 620 | 23 |


| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Lane Group Flow (vph) | 26 | 16 | 0 | 93 | 367 | 0 | 10 | 527 | 107 | 373 | 620 | 23 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |


| Two way Left Turn Lane | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Headway Factor | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |


| Number of Detectors | 1 | 2 | 1 | 2 | 1 | 2 | 1 | 1 | 2 | 1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Detector Template | Left | Thru | Left | Thru | Left | Thru | Right | Left | Thru | Right |
| Leading Detector (ft) | 20 | 100 | 20 | 100 | 20 | 100 | 20 | 20 | 100 | 20 |
| Trailing Detector (ft) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Detector 1 Position(ft) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Detector 1 Size(ft) | 20 | 6 | 20 | 6 | 20 | 6 | 20 | 20 | 6 | 20 |
| Detector 1 Type | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex | Cl+Ex | Cl+Ex | Cl+Ex | Cl+Ex | $\mathrm{Cl}+\mathrm{Ex}$ | Cl+Ex | Cl+Ex |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Detector 1 Queue (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Detector 1 Delay (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Detector 2 Position(ft) |  | 94 |  | 94 |  | 94 |  |  | 94 |  |
| Detector 2 Size(ft) |  | 6 |  | 6 |  | 6 |  |  | 6 |  |
| Detector 2 Type |  | $\mathrm{Cl}+\mathrm{Ex}$ |  | CI+Ex |  | CI+Ex |  |  | CI+Ex |  |


| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Detector 2 Extend (s) |  | 0.0 |  | 0.0 |  | 0.0 |  |  | 0.0 |  |
| Turn Type | pm+pt | NA | pm+pt | NA | pm+pt | NA | Perm | pm+pt | NA | Perm |
| Protected Phases | 7 | 4 | 3 | 8 | 5 | 2 |  | 1 | 6 |  |
| Permitted Phases | 4 |  | 8 |  | 2 |  | 2 | 6 |  |  |

[^23]|  | 4 |  |  | 7 |  |  | 4 | $\dagger$ | P |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Detector Phase | 7 | 4 |  | 3 | 8 |  | 5 | 2 | 2 | 1 | 6 | 6 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 5.0 | 5.0 |  | 5.0 | 5.0 |  | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Minimum Split (s) | 10.5 | 23.5 |  | 10.5 | 23.5 |  | 10.5 | 23.5 | 23.5 | 10.5 | 23.5 | 23.5 |
| Total Split (s) | 10.5 | 23.5 |  | 10.5 | 23.5 |  | 10.5 | 35.0 | 35.0 | 21.0 | 45.5 | 45.5 |
| Total Split (\%) | 11.7\% | 26.1\% |  | 11.7\% | 26.1\% |  | 11.7\% | 38.9\% | 38.9\% | 23.3\% | 50.6\% | 50.6\% |
| Maximum Green (s) | 5.0 | 18.0 |  | 5.0 | 18.0 |  | 5.0 | 29.5 | 29.5 | 15.5 | 40.0 | 40.0 |
| Yellow Time (s) | 3.5 | 3.5 |  | 3.5 | 3.5 |  | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 | 3.5 |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.5 | 5.5 |  | 5.5 | 5.5 |  | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 |
| Lead/Lag | Lead | Lag |  | Lead | Lag |  | Lead | Lag | Lag | Lead | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes |  | Yes | Yes |  | Yes | Yes | Yes | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| Recall Mode | None | None |  | None | None |  | None | Min | Min | None | Min | Min |
| Walk Time (s) |  | 7.0 |  |  | 7.0 |  |  | 7.0 | 7.0 |  | 7.0 | 7.0 |
| Flash Dont Walk (s) |  | 11.0 |  |  | 11.0 |  |  | 11.0 | 11.0 |  | 11.0 | 11.0 |
| Pedestrian Calls (\#/hr) |  | 0 |  |  | 0 |  |  | 0 | 0 |  | 0 | 0 |
| Act Efft Green (s) | 8.1 | 7.8 |  | 11.1 | 8.6 |  | 29.5 | 24.3 | 24.3 | 45.1 | 43.5 | 43.5 |
| Actuated g/C Ratio | 0.12 | 0.11 |  | 0.16 | 0.13 |  | 0.43 | 0.35 | 0.35 | 0.66 | 0.64 | 0.64 |
| v/c Ratio | 0.12 | 0.08 |  | 0.38 | 0.72 |  | 0.02 | 0.80 | 0.15 | 0.68 | 0.52 | 0.02 |
| Control Delay | 28.9 | 19.0 |  | 29.7 | 13.1 |  | 8.8 | 32.6 | 0.5 | 17.5 | 12.4 | 0.1 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 28.9 | 19.0 |  | 29.7 | 13.1 |  | 8.8 | 32.6 | 0.5 | 17.5 | 12.4 | 0.1 |
| LOS | C | B |  | C | B |  | A | C | A | B | B | A |
| Approach Delay |  | 25.1 |  |  | 16.5 |  |  | 26.9 |  |  | 14.0 |  |
| Approach LOS |  | C |  |  | B |  |  | C |  |  | B |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Area Type: | ther |  |  |  |  |  |  |  |  |  |  |  |

Cycle Length: 90
Actuated Cycle Length: 68.5
Natural Cycle: 90
Control Type: Actuated-Uncoordinated
Maximum v/c Ratio: 0.80
Intersection Signal Delay: 18.6 Intersection LOS: B

Intersection Capacity Utilization 65.0\% ICU Level of Service C
Analysis Period (min) 15
Splits and Phases: 2: Parish Ave \& Settler Way


[^24]|  | $\rangle$ | $\rightarrow$ | $\dagger$ |  | 4 | $\dagger$ | $p$ | - | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBL | WBT | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Group Flow (vph) | 26 | 16 | 93 | 367 | 10 | 527 | 107 | 373 | 620 | 23 |
| $\mathrm{v} / \mathrm{C}$ Ratio | 0.12 | 0.08 | 0.38 | 0.72 | 0.02 | 0.80 | 0.15 | 0.68 | 0.52 | 0.02 |
| Control Delay | 28.9 | 19.0 | 29.7 | 13.1 | 8.8 | 32.6 | 0.5 | 17.5 | 12.4 | 0.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 28.9 | 19.0 | 29.7 | 13.1 | 8.8 | 32.6 | 0.5 | 17.5 | 12.4 | 0.1 |
| Queue Length 50th (ft) | 9 | 1 | 35 | 4 | 1 | 163 | 0 | 36 | 71 | 0 |
| Queue Length 95th (ft) | 25 | 15 | 65 | 41 | 8 | \#458 | 0 | \#252 | 419 | 0 |
| Internal Link Dist (ft) |  | 546 |  | 578 |  | 726 |  |  | 498 |  |
| Turn Bay Length (ft) | 200 |  | 80 |  | 180 |  | 230 | 140 |  | 130 |
| Base Capacity (vph) | 209 | 460 | 244 | 697 | 419 | 841 | 840 | 577 | 1231 | 1101 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.12 | 0.03 | 0.38 | 0.53 | 0.02 | 0.63 | 0.13 | 0.65 | 0.50 | 0.02 |

Intersection Summary
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations | \% | F |  | \% | F |  | ${ }^{7}$ | $\uparrow$ | F' | \% | $\uparrow$ | F |
| Traffic Volume (veh/h) | 20 | 2 | 10 | 30 | 8 | 143 | 8 | 485 | 46 | 164 | 570 | 18 |
| Future Volume (veh/h) | 20 | 2 | 10 | 74 | 8 | 318 | 8 | 485 | 88 | 332 | 570 | 18 |
| Initial $Q(Q b)$, veh | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ped-Bike Adj(A_pbT) | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Parking Bus, Adj | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Work Zone On Approach |  | No |  |  | No |  |  | No |  |  | No |  |
| Adj Sat Flow, veh/h/ln | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 | 1870 |
| Adj Flow Rate, veh/h | 26 | 3 | 13 | 92 | 10 | 357 | 10 | 527 | 107 | 373 | 620 | 23 |
| Peak Hour Factor | 0.78 | 0.78 | 0.78 | 0.80 | 0.78 | 0.89 | 0.78 | 0.92 | 0.82 | 0.89 | 0.92 | 0.78 |
| Percent Heavy Veh, \% | 2 |  |  |  | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Cap, veh/h | 138 | 59 | 256 | 450 | 10 | 345 | 268 | 591 | 501 | 414 | 869 | 737 |
| Arrive On Green | 0.03 | 0.19 | 0.19 | 0.06 | 0.22 | 0.22 | 0.01 | 0.32 | 0.32 | 0.16 | 0.46 | 0.46 |
| Sat Flow, veh/h | 1781 | 306 | 1326 | 1781 | 43 | 1548 | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 |
| Grp Volume(v), veh/h | 26 | 0 | 16 | 92 | 0 | 367 | 10 | 527 | 107 | 373 | 620 | 23 |
| Grp Sat Flow(s),veh/h/n | 1781 | 0 | 1632 | 1781 | 0 | 1592 | 1781 | 1870 | 1585 | 1781 | 1870 | 1585 |
| Q Serve(g_s), s | 0.9 | 0.0 | 0.6 | 3.3 | 0.0 | 18.0 | 0.3 | 21.7 | 4.0 | 10.7 | 21.4 | 0.6 |
| Cycle Q Clear(g_c), s | 0.9 | 0.0 | 0.6 | 3.3 | 0.0 | 18.0 | 0.3 | 21.7 | 4.0 | 10.7 | 21.4 | 0.6 |
| Prop In Lane | 1.00 |  | 0.81 | 1.00 |  | 0.97 | 1.00 |  | 1.00 | 1.00 |  | 1.00 |
| Lane Grp Cap(c), veh/h | 138 | 0 | 315 | 450 | 0 | 355 | 268 | 591 | 501 | 414 | 869 | 737 |
| V/C Ratio(X) | 0.19 | 0.00 | 0.05 | 0.20 | 0.00 | 1.03 | 0.04 | 0.89 | 0.21 | 0.90 | 0.71 | 0.03 |
| Avail Cap(c_a), veh/h | 200 | 0 | 364 | 458 | 0 | 355 | 356 | 684 | 579 | 469 | 927 | 785 |
| HCM Platoon Ratio | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Upstream Filter(l) | 1.00 | 0.00 | 1.00 | 1.00 | 0.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Uniform Delay (d), s/veh | 26.4 | 0.0 | 26.5 | 23.9 | 0.0 | 31.4 | 18.9 | 26.3 | 20.3 | 17.2 | 17.3 | 11.7 |
| Incr Delay (d2), s/veh | 0.7 | 0.0 | 0.1 | 0.2 | 0.0 | 56.8 | 0.1 | 12.8 | 0.2 | 18.9 | 2.4 | 0.0 |
| Initial Q Delay(d3),s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| \%ile BackOfQ(50\%),veh/ln | 0.4 | 0.0 | 0.3 | 1.4 | 0.0 | 12.2 | 0.1 | 11.1 | 1.5 | 6.0 | 8.9 | 0.2 |
| Unsig. Movement Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| LnGrp Delay(d),s/veh | 27.0 | 0.0 | 26.6 | 24.1 | 0.0 | 88.2 | 18.9 | 39.1 | 20.5 | 36.0 | 19.7 | 11.8 |
| LnGrp LOS | C | A | C | C | A | F | B | D | C | D | B | B |
| Approach Vol, veh/h |  | 42 |  |  | 459 |  |  | 644 |  |  | 1016 |  |
| Approach Delay, s/veh |  | 26.9 |  |  | 75.4 |  |  | 35.7 |  |  | 25.5 |  |
| Approach LOS |  | C |  |  | E |  |  | D |  |  | C |  |


| Timer - Assigned Phs | 1 | 2 | 3 | 4 | 5 | 6 | 7 | 8 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Phs Duration $(G+Y+R c)$, s | 18.5 | 31.0 | 10.1 | 21.1 | 6.5 | 43.0 | 7.7 | 23.5 |
| Change Period (Y+Rc), s | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 |
| Max Green Setting (Gmax), s | 15.5 | 29.5 | 5.0 | 18.0 | 5.0 | 40.0 | 5.0 | 18.0 |
| Max Q Clear Time (g_c+11), s | 12.7 | 23.7 | 5.3 | 2.6 | 2.3 | 23.4 | 2.9 | 20.0 |
| Green Ext Time (p_c), s | 0.4 | 1.8 | 0.0 | 0.0 | 0.0 | 3.8 | 0.0 | 0.0 |

Intersection Summary

| HCM 6th Ctrl Delay | 39.2 |
| :--- | ---: |
| HCM 6th LOS | $D$ |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 17.8 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | * |  | ${ }^{1}$ | 4 | 7 | ${ }^{1}$ | 4 | F | ${ }^{1}$ | 4 | 7 |
| Traffic Vol, veh/h | 38 | 8 | 26 | 26 | 6 | 56 | 56 | 441 | 26 | 52 | 502 | 52 |
| Future Vol, veh/h | 38 | 8 | 26 | 70 | 6 | 56 | 56 | 483 | 68 | 52 | 546 | 52 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | 150 | - | 150 | 240 | - | 290 | 260 | - | 200 |
| Veh in Median Storage, \# | \# | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 80 | 78 | 79 | 79 | 92 | 80 | 78 | 92 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 49 | 10 | 33 | 88 | 8 | 71 | 71 | 525 | 85 | 67 | 593 | 67 |



HCM LOS F F

| Minor Lane/Major Mvmt | NBL | NBT | NBR EBLn1WBLn1WBLn2WBLn3 | SBL | SBT | SBR |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 928 | - | - | 116 | 84 | 111 | 552 | 969 | - |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds 300s $\quad+$ : Computation Not Defined *: All major volume in platoon

[^25]Synchro 11 Light Report
Page 8

|  | $\stackrel{ }{*}$ | $\rightarrow$ |  | 7 |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | $\hat{F}$ |  |  | ${ }_{\text {¢ }}$ |  | ${ }^{7}$ | $\hat{1}$ |  | ${ }^{7}$ | $\hat{1}$ |  |
| Traffic Volume (vph) | 6 | 90 | 0 | 0 | 92 | 29 | 0 | 0 | 0 | 15 | 0 | 2 |
| Future Volume (vph) | 48 | 90 | 0 | 0 | 92 | 57 | 0 | 0 | 0 | 44 | 0 | 46 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 150 |  | 0 | 0 |  | 0 | 90 |  | 0 | 90 |  | 0 |
| Storage Lanes | 1 |  | 0 | 0 |  | 0 | 1 |  | 0 | 1 |  | 0 |
| Taper Length (ft) | 25 |  |  | 25 |  |  | 25 |  |  | 25 |  |  |
| Lane Utill. Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  |  |  |  | 0.947 |  |  |  |  |  | 0.850 |  |
| Flt Protected | 0.950 |  |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1770 | 1863 | 0 | 0 | 1764 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Flt Permitted | 0.950 |  |  |  |  |  |  |  |  | 0.950 |  |  |
| Satd. Flow (perm) | 1770 | 1863 | 0 | 0 | 1764 | 0 | 1863 | 1863 | 0 | 1770 | 1583 | 0 |
| Link Speed (mph) |  | 30 |  |  | 30 |  |  | 30 |  |  | 30 |  |
| Link Distance (ft) |  | 716 |  |  | 677 |  |  | 727 |  |  | 714 |  |
| Travel Time (s) |  | 16.3 |  |  | 15.4 |  |  | 16.5 |  |  | 16.2 |  |
| Peak Hour Factor | 0.78 | 0.82 | 0.78 | 0.78 | 0.82 | 0.79 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 | 0.78 |
| Adj. Flow (vph) | 62 | 110 | 0 | 0 | 112 | 72 | 0 | 0 | 0 | 56 | 0 | 59 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 62 | 110 | 0 | 0 | 184 | 0 | 0 | 0 | 0 | 56 | 59 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 60 | 60 |  | 9 | 60 |  | 60 | 15 |  | 9 |
| Sign Control |  | Free |  |  | Free |  |  | Stop |  |  | Stop |  |

## Intersection Summary

Area Type: Other

Control Type: Unsignalized
Intersection Capacity Utilization 16.6\% ICU Level of Service A

Analysis Period (min) 15



[^26]Synchro 11 Light Report Page 10

## MOUNTAIN VIEW WEST P.U.D.

## Design Guidelines

Parish LLC
8714 State Highway 60
Johnstown, CO 80534
Developer


November 2017

## TABLE OF CONTENTS

# *NOTICE TO APPLICANTS, DEVELOPERS, BUILDERS, BUYERS, TENANTS AND OTHER OCCUPANTS OF MOUNTAIN VIEW WEST SUBDIVISION...THESE MOUNTAIN VIEW WEST GUIDELINES SHOULD BE USED IN CONNECTION WITH OTHER TOWN OF JOHNSTOWN ADOPTED PLANS, REGULATIONS AND STANDARDS, INCLUDING, BUT NOT LIMITED TO: 

JOHNSTOWN'S MUNICIPAL CODE (AS IT MAY BE AMENDED)<br>JOHNSTOWN'S AREA COMPREHENSIVE PLAN NOVEMBER 2006<br>JOHNSTOWN'S DESIGN GUIDELINES, AS AMENDED<br>JOHNSTOWN'S ZONING AND SIGNAGE CODES<br>JOHNSTOWN'S TRANSPORTATION PLAN FEBRUARY 2008<br>JOHNSTOWN'S CRITERIA AND CONSTRUCTION REGULATIONS APRIL 2004<br>JOHNSTOWN/MILLIKEN PARKS, TRAILS, RECREATION AND OPEN SPACE PLAN MAY 03<br>JOHNSTOWN'S LANDSCAPE STANDARDS AND SPECIFICATIONS 2004<br>ANNEXATION AGREEMENT DATED APRIL 7, 2014<br>MOUNTAIN VIEW WEST MASTER AND RESIDENTIAL HOA COVENANTS (CC\&Rs)<br>MOUNTAIN VIEW WEST PLAT NOTATIONS<br>mountain view west development agreement and exhibits thereto

Guideline Sections within these MVW Design Guidelines include the following Sections;

### 1.0 Introduction

2.0 Purpose and Intent of these Guidelines, Vision and Description
3.0 Proposed Land Uses, Approval Committees and Processes, Additional Criteria, Variances, Final Plan Amendments, CC\&Rs and JRC Acknowledgement by Council
4.0 Architectural Design Guidelines In General for the MVW Subdivision
5.0 Guidelines Specific for each Planned Land Use Including Single and Multifamily residences, Commercial Office, Commercial Retail and Special Commercial as well as Flex Space, Light Industrial and Xeriscape Landscaping
6.0 Open Space and Trails
7.0 Signs
8.0 Streetscapes, Furniture and Street Lighting
9.0 Storm Water
10.0 Utilities, Easements and Rights of Way
11.0 Grading
12.0 Screening Walls and Use of Berms
13.0 Emergency Access
14.0 Parking Lots, Transportation and Transit Stops
15.0 Irrigation
16.0 Irrigation Maintenance
17.0 Bicycles
18.0 Definitions

Exhibits A-C

### 1.0 Introduction

In accordance with the Johnstown Area Comprehensive Plan, Mountain View West (MVW) subdivision is a covenant controlled master-planned community that is located in the northeast corner of the intersection of Parish Avenue (WCR 17) and Centennial Drive (WCR 46 1⁄2). The subdivision located just south of Johnstown's existing downtown is to provide an extension to the existing downtown corridor of Johnstown by extending both commercial and residential uses blended within a harmonious mix of neo traditional craftsmen style designs throughout the water wise subdivision.

### 2.0 Statement of Purpose and Intent of these Guidelines

The purpose and intent of these guidelines is to provide design guidelines for developers, builders and property owners by establishing timeless design guidelines and concepts that maintain the unique character planned for MVW by Parish, LLC in creating a "community within a community" providing MVW residents and business owners a feeling of pride that they have invested in a unique place in which to reside, work, play and shop all within easy walking distance of the existing downtown, and near many Town service facilities. MVW is extending the existing downtown corridor southerly to Centennial. These guidelines will help to insure that the unique character and intent planned for MVW is carried out by future developers and builders choosing to build within MVW assuring consistent design elements and characteristics are maintained throughout the MVW community. These guidelines along with other documents referenced herein provide a basis to ensure that the character of MVW is maintained throughout the subdivision, providing overseers consistency on acceptable site planning, landscaping, streetscapes, parking, signs and signage, and architecture on an ongoing basis. The guidelines also ensure MVW residents and occupants a feeling they are safe and secure in their living and working environments while preserving real estate values in a comfortable community reminiscent of days gone by.

### 2.1 The Vision for Mountain View West

MVW is located in close proximity to the Johnstown Downtown Corridor. As such MVW will have extra wide sidewalks along Parish Avenue to encourage walkability to and from the downtown. The retail office areas will have excellent curb appeal from Parish whether visitors are coming to the center by foot, bicycle or motorized vehicle. Throughout the subdivision MVW is designed to incorporate neo traditional craftsman styled architecture throughout the community keeping in character with the roots of Johnstown. Residential areas will display tree lined streets and front porches will be encouraged within the architectural guidelines for residential areas featuring moderate to high densities as called out in the Town's Design Guidelines. The entire MVW community will be under landscaping guidelines that are based upon xeriscape landscape techniques and plant species providing for a water wise community. Xeriscaping landscape guidelines are a part of these guidelines as is a list of suggested planting materials.

### 2.2 Mountain View West Site Description

MVW is located in the northeast corner of the intersection of Parish Avenue (WCR 17) and Centennial Drive (WCR $461 / 2$ ) and contains approximately 62 acres of land area. The site is gently sloping from the northwest to the southeast, where the subdivision's detention pond is planned. From the detention pond water will be released into the Little Thompson. The land has been used for agricultural uses for many decades. The land was annexed into the Town on or about April 7, 2014 and has been entitled as a Planned Unit Development (PUD). MVW is bounded on the west by Parish Avenue; on the south by Centennial Drive; on the east by Great Western Railway right of way; and on the north by The Colony and land owned by Weld County.

### 3.0 Proposed Land Uses



MVW is a Planned Unit Development that will generally consist of approximately 30 acres of commercial office, employment and retail uses and 32 acres of residential uses. MVW is a phased subdivision with two phases. Phase I encompasses approximately 32 acres to the north of the planned easterly extension of Settler's Way. Phase II will contain approximately 30 acres lying south of the easterly extension of Settler's Way. Some utility connections and the MVW subdivision's storm water detention facility are located within the boundary of Phase II, but will be completed as part of the Phase I improvements. Several land uses have been identified as appropriate uses at this time for MVW and they are listed herein. Phase I will have 11 acres of commercial property including retail, office and employment uses that may be located within flex space. Phase I may also accommodate up to 17 acres of residential land uses. In accordance with Goal DT 4 of Johnstown's Design Guidelines MVW residential will be planned for 8 to 10 dwelling units per acre. Residential and commercial areas within MVW will be complementary. Wide sidewalks with street trees and benches and public art displays will guide and invite people from downtown Johnstown to MVW. Phase II of MVW will contain an additional 24.46 acres of land area. Commercial and Special Commercial uses will occupy 13.81 acres of Phase II with medium density residential uses taking up the additional 10.65 acres. In those cases where these guidelines are in conflict with the Town of Johnstown standards and regulations
within the Town's Design Guidelines, then the Town's Design Guidelines shall prevail. The provisions of these MVW Guidelines shall supersede any conflicting provision(s) of the then prevailing Johnstown Municipal Code and may only be modified to protect the health, safety, and welfare of the general public by the Town's Council following at least thirty (30) days written notice to the record owner(s) of real property that will be affected by the intended modification.

### 3.1 GENERAL PROCEDURES FOR SUBMITTALS AND APPROVALS

MVW shall establish a Design Review Committee (DRC) in order to assure that all of the MVW Master and Residential Home Owner Association (HOA) Covenants, Conditions \& Restrictions (CC\&Rs) design standards are followed and adhered to before a final plan is submitted to the Town of Johnstown for administrative review and approval by the Johnstown Review Committee (JRC). An applicant shall first start with discussing its planned project with the DRC.

### 3.1.1 Design Review Committee

The Design Review Committee is established to ensure that all proposed development projects to be constructed meet the standards established in these MVW Design Guidelines. Guidelines have been established to ensure consistency with character and design throughout the MVW community. These MVW Design Guidelines have been promulgated and adopted by MVW and the Town of Johnstown for the sole purpose of providing land use regulations which will form the basis for decisions made by the DRC as well as the JRC as they review all planned project that are submitted to them for review and approval. The DRC and JRC will review and approve all site, building and landscape plans for the MVW subdivision. Planned projects that do not meet these adopted MVW Design Guidelines will not be approved. The DRC will be made up of an architect or engineer, a landscape architect and a representative of the subdivision's Owner. The DRC shall meet regularly to review plans submitted to it, but shall only review submittals that are complete with all required documents submitted as required by the DRC. Applicants are encouraged to have pre-application meetings with the DRC or its members.

### 3.1.2 Design Review Committee Approval Process

Any time a party wished to build, demolish, or substantially modify an improvement within MVW that party must first have their plans approved by the DRC prior to commencement of any work related to such construction, demolition or modification. The party must thereafter also receive a similar approval from the JRC before commencement of work. Items under the purview of the DRC include, but are not limited to, building elevations, site plans, site photometric plans, site engineering, landscape designs, signage, and other similar items as identified by the DRC. The DRC will require applicants to submit complete packages for DRC review at least 10 days prior to scheduled DRC meeting so that the submittal documents can be dispersed to DRC members for their review prior to the meeting date. Submittals shall be made with electronically in a pdf format, unless otherwise directed by the DRC. Submittals shall not be considered complete unless and until any required submittal fee is received by the DRC. A formal presentation to the DRC may be requested at any time by the DRC by providing the applicant advance notice. Upon notice to the applicant that the DRC has approved the applicant's submittal the applicant may submit the project to the JRC, so long as the submittal documents are consistent with those that were approved by the DRC with no changes or modifications thereto.

### 3.1.3 Johnstown Review Committee (JRC)

The Johnstown Review Committee may be made up of the Town's Planner and Town Manager, or other professionals engaged by the Town. The JRC shall review the documents that have been previously approved by the DRC that are thereafter submitted to the JRC in their exact same format without any changes or modifications. All Town of Johnstown building codes,
subdivision regulations, fees and permits as adopted from time to time by the Town shall apply. See NOTICE TO APPLICANTS above for additional controlling documents.

### 3.1.4 Johnstown Review Committee (JRC) Approval Process

Applicants after having their project reviewed and approved by the DRC must then have their application approved by the JRC, after it has been reviewed and approved by the DRC. The applicant shall submit their project to the JRC pursuant to the following approval process:

1. Pre-application Discussion

Applicants may and are encouraged to schedule a pre-application meeting with the Town Planner for Johnstown to informally discuss and review the applicant's planned use of the site in question. This shall include the applicant's interpretation of the MVW Design Guidelines as they relate to the applicant's project. Review of a sketch plan along with elevations and other items as have been addressed and approved by the DRC will be instrumental in assuring good communication of the intended use.
2. Final Development Plan Submittal and Process

Projects being submitted to the JRC as a Final Development Plan Submittal shall be made on the appropriate forms accompanied by the appropriate fees as charged by the Town. A letter from the DRC should accompany the submittal indicating that the applicant has received the approval of the DRC. The Town will review the submittal within seven (7) days of it being submitted for completeness of the submittal. If the Town deems the submittal to be complete the JRC will review the applicant's project submittal. If the submittal is deemed incomplete then the JRC shall inform the applicant in writing as to how the applicant can modify the submittal to make it complete. Once the JRC has a complete submittal for review it shall make its determination within forty five (45) days of the submittal date as to whether or not the application is in conformance to the MVW Design Guidelines as adopted. The JRC may elect to grant variances to the applicant upon the DRC's recommendation and the applicant's ability to provide and demonstrate to the JRC a better design solution. The Town however will not be able to grant a variance to a permitted use.
3. Johnstown Review Committee Approval
4. Once the JRC has determined that the applicant has complied with all of the MVW Design Guidelines, as well as other Town of Johnstown controlling documents and regulations, the JRC may grant its approval for the applicant's project. The JRC shall approve the application if it complies with the applicable terms and conditions of the MVW Guidelines and other Town guidelines, rules and regulations. The JRC may approve the application with conditions. Said conditions shall be specifically related to compliance with standards and guidelines as listed herein. In the event that the JRC determines that the proposed development in the application does not comply with the Design Guidelines, the JRC shall specify in writing the specific reasons in which the application does not meet applicable criteria.
5. Johnstown Review Committee Appeal

The decision of the JRC may be appealed by the applicant to the Johnstown Town Council. The appeal shall be in writing, and shall be made within thirty (30) days of the date of the transmittal of the JRC's decision. The Johnstown Town Council shall hear the appeal within thirty (30) days of the filing of the appeal by the applicant. The decision of the Johnstown Town Council shall be final regarding the applicant's appeal.
6. Resubmittals: Resubmittals of applications that required modifications be made will be processed in the same manner and within the same time frame as the initial
application as shown in the JRC Approval Process 3.1.4 in paragraphs number 1 through 4 above.

### 3.1.5 ADDITIONAL CRITERIA AND UPDATES

In addition to the criteria herein the DRC and JRC may promulgate additional criteria that not inconsistent with the criteria set forth herein. From time to time, any of these additional criteria may be amended by action of the DRC and JRC. Change in land use or changes greater than the $20 \%$ dimensional criteria, that shall become a permanent part of the design guidelines document shall constitute a major change and shall be brought back to the Planning Commission and Town Council for review and approval.

### 3.1.6 VARIANCES

The DRC may authorize a variance to the MVW Design Guidelines when circumstances such as topography, natural obstructions, hardship, or aesthetic or environmental objectives or considerations may warrant, insofar as they are not superseded by applicable Town of Johnstown zoning regulations. Such variances must be approved by the DRC and JRC. A variation of up to $20 \%$ in a dimensional standard is allowed if it improves the project design or an unreasonable hardship can be demonstrated by the applicant.

### 3.1.7 FINAL PLAN AMENDMENTS

Amendments to Final Plan Documents must be approved in writing by both the DRC and JRC.

### 3.1.8 COVENANTS, CONDITIONS \& RESTRICTIONS (CC\&Rs)

MVW will submit to the Town of Johnstown at the time of recording of the Final Plat a complete copy of CC\&Rs for MVW for review and recording. The CC\&Rs may address other items that are not addressed within these MVW Design Guidelines. However, each and every covenant, condition and restriction within the CC\&Rs shall be subordinate to the MVW Design Guidelines. CC\&Rs shall conform to all State of Colorado statutes and regulations as well as any that may be established by local governmental bodies or the Town of Johnstown.

### 3.1.9 JRC APPROVAL OF GUIDELINES ACKNOWLEDGED BY COUNCIL

The Johnstown Review Committee (JRC) acknowledgement and approval of these Mountain View West (MVW) Design Guidelines shall be acknowledged and approved by the Johnstown Town Council by Resolution of the Town Council.

### 4.0 ARCHITECTURAL DESIGN GUIDELINES IN GENERAL

The architectural guidelines provided within this section provide specific design features and elements that MVW want to achieve in its overall design of a quality subdivision that is a new southerly extension to the downtown corridor of Johnstown. MVW intends to maintain the charm and character of the older parts of the Johnstown community and develop a new residential and activity center for retail, office and employment sectors within the community. The architectural style that has been selected by Parish LLC is the neo traditional craftsman style which is close in design to much of the existing buildings and homes located in the core area of Johnstown. Because MVW is only 62 acres in size and is located along Parish Avenue across from the Johnstown municipal complex, it is developing a large portion of the acreage, approximately 30 acres in total for commercial, office, health care and retail uses with the balance of the property developing as residential dwelling units at the rate of 8 to 10 units per acre as indicated in the Johnstown Design Guidelines as Downtown Goal \#4. There will be approximately 32 acres of medium density residential dwellings constructed at MVW in several smaller communities, each with their own identity and some being age restricted communities. Temporary construction, sales and leasing offices of a free standing nature, for periods of less than one year, shall also
be allowed as an accessory use in MVW. Recreational uses shall be considered an allowable accessory use in MVW. Any other structure or use clearly incidental to or that is generally and commonly associated with the operation of any permitted use that is permitted within these MVW Design Guidelines.

Design goals and objectives at MVW shall include:
Attractive separation from neighboring projects
Buildings with multiple sided architecture not just street side architecture Buildings located with interesting orientations in commercial areas
Trash enclosures that are fully screened so that dumpsters are not visible
Mailboxes will be located in well lighted accessible and safe areas
Regional materials should be encouraged and used as much as possible
Adjoining properties are encouraged to share access points and allow circulation Projects need to provide for bicycle parking and safe circulation through the site Routes shall be clearly delineated, visible and marked for safety
Safe zones for pedestrians at all intersections with vehicles
Open and unobstructed sight triangles
Roof mounted objects should be screened or place away from street sides Lighting levels throughout MVW commercial areas shall be subdued, not bright Use of building based lighting is encouraged
Where fences are used they should be open in nature
Privacy Fencing - Allowed but not in linear runs more than 20 feet in 1 direction
Drive thru restaurant service lanes shall be screened or bermed
Drive thru service menu boards shall not be visible to public streets
Enhancement of the scale and style of the central business district is encouraged

### 5.0 ARCHITECTURAL DESIGN GUIDELINES RESIDENTIAL

The submittal process for single family or multifamily residential projects in MVW will have an abbreviated submittal process as compared to commercial, office, retail, and flex projects, but shall parallel the process described in 3.1 to 3.1.7 above. Builders and owners should ask the MVW Residential HOA for details and proper application forms. Whether a project is being constructed as a residential or commercial project these MVW Design Guidelines will control and shall be enforced by the ARC and JRC. Home occupations shall be allowed within the residential areas of MVW subject to any restrictions placed upon such use by Johnstown.

### 5.1 ARCHITECTURAL DESIGN GUIDELINES SINGLE FAMILY RESIDENTIAL

Phase I of MVW may not have any detached single family housing as it is currently planned for medium density attached single family residences. See attached Exhibit A to the MVW Design Guidelines for more information and samples of acceptable Craftsman Style architecture to be constructed at MVW. Acceptable accessory uses would include garages, gazebos, gas grills, and patios. No storage sheds will be allowed that are not an attached part of the residence and designed to be compatible with the architecture of the residence.

### 5.2 ARCHITECTURAL DESIGN GUIDELINES MULTIFAMILY RESIDENTIAL

Multifamily dwellings will meet the same Craftsman Style architecture as described within the MVW Design Guidelines attached as Exhibit A to these Guidelines. Multifamily dwelling density shall range from 12 to 16 dwellings units per acre. At least one garage shall be available for occupants of the multifamily dwelling units, plus additional onsite parking as is required by the Johnstown Municipal Code. Acceptable accessory uses include garages and landscape and site features such as gazebos and private and shared outdoor patios and grill areas and hot tubs.

Maintenance structures shall also be a permitted accessory use as well as patio or deck storage units attached to the buildings.

### 5.3 ARCHITECTURAL DESIGN GUIDELINES COMMERCIAL OFFICE AND FLEX SPACE

Office buildings and hybrids such as flex space incorporating office and some lab space or inside storage or work space shall also meet similar Craftsman Style architecture so as to be compatible with MVW residences. Uses in these areas will include service businesses such as banks and medical and dental offices as well as standard professional office uses. Skilled care nursing facilities, independent living structures, assisted care living structures and other similar specialty housing types will also be allowed within this area. See attached Exhibit B to the MVW Design Guidelines for a depiction of the architectural style and design elements that are to be incorporated into the designs of office buildings and flex space at MVW. Acceptable accessory uses will include outdoor patios or other gathering areas, free standing signs, parking garages and other similar items that would be customary to these type facilities.

### 5.4 ARCHITECTURAL GUIDELINES RETAIL COMMERCIAL \& SPECIAL COMMERCIAL

MVW considers that uses within these definitions if a Convenience Center and would include, but not be limited to, retail sales, service businesses, restaurants and restaurants with drive-thru lanes, bakeries, coffee shops, drug stores, food stores, specialty food stores, work-out gyms, laundromats, drug stores dry cleaners, hardware stores and many medical service facilities including emergency clinics, delis, salons and repair shops of all kinds. Many other similar businesses will fit into this same category, including brew pubs and convenience stores with gasoline sales. See attached Exhibit B to the MVW Design Guidelines for a depiction of the architectural style and elements to be incorporated into the designs of retail commercial and special commercial buildings at MVW. Acceptable accessory uses will include outdoor patios or other gathering areas, free standing signs, parking garages and other similar items that would be customary to these type facilities. Outdoor seating, benches and patio furniture shall also be allowed to provide gathering places.

### 5.5 ARCHITECTURAL DESIGN GUIDELINES LIGHT INDUSTRIAL OFFICE FLEX SPACE

MVW will accept certain clean assembly, processing and fabrication facilities, as well as printing and publishing businesses, but these kinds of business use should not be fronted out on Parish Avenue. Buildings housing these types of uses shall also be designed to fit with the Craftsman Style architecture as it is depicted on Exhibit B to these MVW Design Guidelines. Acceptable accessory uses will include outdoor patios or other gathering areas for employees, free standing signs, parking garages and other similar items that would be customary to these type facilities. Many of these same users could fit into flex space.

### 5.6 ARCHITECTURAL DESIGN GUIDELINES ADDITIONAL CRITERIA

In addition to the criteria set forth herein, the DRC and JRC may promulgate additional criteria from time to time that are not inconsistent with those that are set forth herein. Any of the new or additional criteria may be amended by action of the DRC and JRC. Changes in land use or changes to any criteria that is greater than the 20 percent dimensional criteria that shall become a permanent part of the design guideline document, shall constitute a major change and shall be brought back to the Planning Commission and Town Council for review and approval.

### 5.7 ARCHITECTURAL DESIGN GUIDELINES LANDSCAPING

MVW will be a water wise subdivision and has designed a complete set of xeriscape landscape regulations, including suggested planting materials for MVW. These Xeriscape Landscape Plans and Plant List are attached as Exhibit C to these MVW Design Guidelines.

### 6.0 Open Space and Trail

MVW dedicated open space at the time of its annexation into the Town of Johnstown that filled it requirement for open space dedication by providing a strip of land seventy five (75) feet in width either side of the Little Thompson River from the center line of the stream from the east side of the Great Western Railroad south all the way to WCR 19. The Town of Johnstown plans to improve this area with walking and biking paths leaving the bulk of the area open as a natural area. The large detention pond located in the southeast corner of MVW will be graded such that the bottom of the pond can be used for recreational activities when the pond is dry. Connections will be made from MVW to the Little Thompson open space trails when that area has been improved.

### 7.0 SIGNS

MVW will have two entry monument signs, see streetscape plans Section 8.1. One shall be located at the entrance to MVW off of Parish Avenue at the Settlers Way entrance in the southeast corner of the entrance. The other will be located along Centennial Drive at the future intersection of Mountain Bluebird Drive that is planned for Phase II of MVW. The entry monument signs will include native stone, a lighted sign and xeriscape landscaped areas with the addition of seasonal flowering plants during the summer months. Throughout the MVW subdivision all signage wherever possible shall be of a more human scale with lighted ground based monument signs used to identify tenants and users of commercial, office, flex and light industrial buildings and sites. Monument signs will be located in easements set aside for such. Façade signs will be allowed on retail buildings with back lit pan channel letters all sized in conformance with Town of Johnstown sign standards. Banners shall be allowed, but shall not be allowed to be displayed for more than one 7 day period of time during each 6 month period during a calendar year, those periods being January through June and July through December each year. Allowable sign areas and sizes shall be as allowed per the Town of Johnstown's Sign Code, at the time that the MVW Design Guidelines are adopted.

### 8.0 STREETSCAPES, FURNITURE \& LIGHTING

### 8.1 STREETSCAPES

MVW will have tree lined streets throughout using a mixture of deciduous trees and ornamental trees as well as coniferous varieties and evergreen shrubs. That area along Parish Avenue shall be designed to have a blend of all kinds of plant materials mentioned herein, but MVW will focus mostly on xeriscape varieties of plants and trees. The roundabout at the junction of Settlers Way and Mountain Bluebird Drive will also be heavily landscaped, but not so that sight line views are obstructed.


MVW commercial lots on Parish Avenue have the possibility to continue the downtown theme.





### 8.2 STREET FURNITURE \& STREET LIGHTING

At strategic locations along Parish Avenue and elsewhere throughout MVW along local streets decorative benches and paving and other outdoor furnishings will be strategically placed. Street lighting internal to MVW will be of a decorative nature and street lighting along Parish Avenue shall mirror that


Site furniture examples from Johnstown's downtown. used in the downtown area of Johnstown. The visual character of the drive or walk southerly along Parish Avenue will provide a sense of continuity to the downtown.


Examples of existing site \& street lighting nearby.


### 8.3 FENCING \& WALLS

All fencing should complement the project's architecture. Any walls should, in as much as possible, match the architecture of the project's buildings. Any fencing and walls shall be subject to JRC and DRC.


Examples of possible fencing styles.

### 9.0 STORM DRAINAGE AND DETENTION POND

The goal of each site design within MVW shall be to minimize runoff, to the subdivision's storm drainage pond located in the southeast corner of the subdivision which has been designed such that no on site detention or retention is required. MVW storm drainage pond and all calculations have been based upon the most recent data available from the proper authorities so that water being returned to the river is improved via the most recent technology to improve water quality. All sites and lots within MVW shall be designed to minimize the amount of storm water that goes from one site or lot onto a neighboring site or lot. Wherever possible water from parking lots and roof drains shall be directed toward and utilized by landscaped areas, reducing the need for irrigation water. Drainage water shall flow along driveways, dedicated street flow lines, swales and landscaped corridors on their way to the MVW detention pond where it will be introduced into the Little Thompson. Screen grates and trash grates shall be installed at outflow structures.

## 10. UTILITIES, EASEMENTS AND RIGHTS OF WAY

All existing easements and rights of way for existing utilities have been identified and located on the plat to be filed for MVW. MVW will also be dedicating certain utility easements and rights of way for future development of sites and lots throughout the subdivision. Developers and Builders shall not interfere with any such easements and rights of way and it shall be their sole responsibility to be aware of all such easements and rights of way on their site or lot prior to commencement of construction activity. Landscaping placed over the top of any easement or right of way is subject to future destruction and shall be the sole cost of the site or lot owner for any replacement.

## 11. GRADING

All construction activity on all sites and lots in MVW shall be constructed to provide positive drainage away from buildings and foundations, but not over sidewalks. No grading shall take place outside of the owner's site or lot without the proper authority from the neighboring land owner or Town when appropriate.

## 12. SCREEN WALLS AND USE OF BERMS

Within MVW all above ground utility boxes, meter banks, loading areas, and outside equipment storage areas shall be screened by a screen wall that matches the architectural nature of the adjacent building using the predominant materials and colors of the building or by the use of a landscaping berm. Screen walls and berms shall minimize the visual impact of the items being screened by at least seventy five percent (75\%) from view of adjacent streets.

## 13. EMERGENCY ACCESS

All MVW site and street designs shall provide for safe and expeditious access for police, fire, ambulances and other emergency vehicles to residences and commercial buildings in line with the regulations set forth from time to time by the Johnstown Fire Department.

## 14. PARKING LOTS, TRANSPORTATION AND TRANSIT STOPS

As Johnstown continues to grow transportation will become an ever increasing topic of local conversation. To stay ahead of transportation issues MVW designs will allow for transit stops to be accommodated with retail and commercial areas of MVW as the need arises. In the interim most local residents will likely be using their vehicles when not walking or biking to MVW or within MVW to its commercial areas and tenant occupants. Parking lots in commercial areas will be designed to Town of Johnstown design criteria and construction regulations. Within all MVW residential areas it is anticipated that residences will have at least one parking garage with most having two parking garages. Apartments may not have enough covered parking in garages for all apartments, but they shall still meet the minimum parking requirement of 1.5 parking spaces per one bedroom dwelling and 2 parking spaces per two bedroom dwelling unit and 2.5 parking spaces per three bedroom dwelling unit. Commercial offices shall be designed with 1 parking space per 300 SF of gross leasable area and Commercial retail shall be designed with 1 parking space per 250 SF of gross leasable area. Medical offices shall be designed with 1 parking space per 200 SF of gross leasable area. Restaurants shall be required to have I parking space for each 100 SF of gross leasable area. Other uses not mentioned herein shall be as directed by the Town Johnstown Design Guidelines. All parking lots will be designed to current standards of designed to current Americans with Disabilities Act (ADA) standards. Interior rows of parking spaces will provide a landscape island at the end of each row of parking, and landscape islands will be provided within the row of parking spaces so that there is not more than 20 consecutive parking spaces without a minimum 9 foot wide landscape island separating the parking spaces. Bicycle parking shall be prominently made available throughout MVW within the commercial and office and retail areas. Bicycle parking areas shall be well lit to provide a safe environment for cyclists.

## 15. IRRIGATION AND IRRIGATION SYSTEMS

All landscaped areas at MVW shall have irrigation systems operated by time clocks which shall be operated at non high volume times, generally during the night time hours and set to turn off just before sun rise to minimize evaporative losses. Landscaped areas such as large urns and pots bearing live plant material will require hand irrigation if not set up with automatic sprinkler systems. Irrigations systems will be designed with adequate zones to minimize irrigation tap sizes. The irrigation line shall have an automatic controller to activate and operate the system. Remote control valves shall operate each zone valve. Patterns of sprinkler heads will be set to provide head-to-head coverage to all landscaped areas. The system operator shall manage the system so that no the sprinkler do not spray or irrigate impervious surfaces, including sidewalks, driveways, streets and parking lot areas. Backflow prevention devices shall be installed on all irrigation systems. Please see Exhibit C to these MVW Design Guidelines for further explanation of the xeriscape landscaping to be used at MVW and the water conservation benefit by the use of xeriscape methods and planting materials. MVW is a water wise xeriscape subdivision and as such all landscaped area shall be designed as per the Exhibit C Landscape Design criteria to these MVW Design Guidelines.

## 16. IRRIGATION MAINTENANCE

Maintenance of irrigation shall include all reasonable and regular irrigation, weeding, weed control, fertilizing, pruning, timely removal of tree wraps and staking, and bike path snow and ice removal per usual and standard horticultural practices and Town of Johnstown code. All plant
materials that show signs of insect infestation, diseases or other damage shall be appropriately and timely treated. Dead plant material will be replaced according to the approved landscape plan for MVW and the particular site or lot within MVW. An initial inspection of the landscaping installation will be completed at the time of completion of construction or at any time when there is a change in use. The original developer and any subsequent owner(s) shall be responsible for maintaining all on-site and common areas landscaping as shown on the approved landscape plan for the site or lot. MVW through its Master Association and Residential Association(s) shall be responsible for maintaining the landscaping of public improvements on all adjacent rights-ofway as shown on the approved landscape plan unless a maintenance agreement is existing with a third party. The Town, at its discretion, may add, remove, replace, or maintain landscape materials within any right of way per Town of Johnstown standards.

## 17. BICYCLES

Bicycles have become common place in today's society whether it be for recreation, work or for shopping. MVW recognizing this fact shall endeavor to accommodate bicycle riders within all commercial areas by providing bicycle routes and by providing adequate bicycle parking spaces in all commercial areas within MVW. Bicycle parking facilities shall be located to provide safety, security and convenience for bicycle riders. Such bicycle facilities shall not interfere with, and be located a safe distance from, pedestrian and motor vehicular traffic. It is highly recommended that bicycle parking facilities be designed and constructed to allow the bicycle frame and both wheels to be securely locked to the bicycle parking structure. The structure shall be of a metal or other permanent construction material and permanently attached to a concrete foundation.


Examples of possible parking device styles.

## 18. DEFINITIONS

1. Animated sign - A moving sign that utilizes motion in a horizontal or vertical plane or both.
2. Berm - An undulation in terrain creating a new landform within a landscape to be utilized for wind protection, screening or a point of focal interest.
3. Building - Any structure used, designed or intended for the roofed shelter, enclosure or protection of persons, animals or property.
4. Clinic...Medical, Dental or Other - Offices organized to provide medical, dental or other types of health services and/or supplies
5. Convenience Center - A small group of retail stores and service establishments which serve the local neighborhood, including, by way of example but not of limitation, a food store, drugstore, hardware store, barber shop, beauty salon, restaurant, shoe repair shop or laundromat.
6. Development - A single lot, parcel or tract of land or portions or combinations of lots, parcels or tracts of land which are held in single or common ownership and which exist as a distinct functional entity. Multi-use buildings and multiple building complexes which
are held in singular or common ownership, either by individuals, corporations or other legal entity, shall be considered a development for the purpose of the MVW Desgin Guidelines.
7. DRC - The MVW Design Review Committee
8. Flashing Sign - A sign that is illuminated with intermittent lighting, animated lighting or with varying intensities of light at intervals of fifteen (15) seconds or less, including a moving light or lights.
9. Flex Space - Flex space lends itself to multiple uses which is described by its name. A flex space building is designed for multiple tenants, divided in spaces generally running from front to back. Office space is usually located at the front of the building with other space to the rear that can be used for warehouse space, or assembly space that is typically accessed by delivery doors at the rear of the building. Flex space may include such uses for offices, retail, wholesale, warehousing, manufacturing, assembly, light industrial, or research and laboratory facilities, with residences on second floors.
10. Freestanding Sign - Also referred to as a ground sign. A sign that is permanent and selfsupporting, being non-dependent upon support from a building or other type of structure, including signs placed upon fences or non-supporting walls.
11. Gross Floor Area - The total floor area of a commercial building that is inhabitable by the building's occupant or multiple occupants if the building is divided or divisible.
12. Gross Leasable Area (G.L.A.) - The total floor area of a commercial building, which floor area is designed for a tenant or tenants' occupancy and exclusive use, including basements, mezzanines and upper stories, expressed in square feet and measured from the center line of joint partitions and from outside wall faces.
13. Home Occupations - The legal use and occupation of a home, where permitted as an accessory use, for the purpose of doing business out of the home. Such use shall not change the character of the home and the home shall not be allowed exterior signage to promote such home occupation and not external storage shall be allowed. The home occupation shall not create any offensive noise, vibration, smoke, dust, odors, heat or glare noticeable to other area occupants.
14. JRC- The Johnstown Review Committee
15. Lot - A single parcel of land occupied or intended to be occupied by such structure or structures and uses as may be permitted by zoning.
16. Lot Area - The area of contiguous land bounded by lot lines, exclusive of land provided for public thoroughfare.
17. Lot Lines - The lines bounding a lot as defined above.
18. Office or Professional Office - The office of a doctor, dentist, architect, landscape or other architect, engineer, attorney or other similar recognized profession.
19. Open Space - The gross area of a lot or tract of land minus all streets, driveways, parking lots, and building areas, which is to be or has been landscaped or developed for use by the public or by the residents of the lot or tract of land for private, common or public enjoyment or recreational use.
20. Retail Store - A commercial establishment for the sale of material goods or commodities in relatively small quantities selling directly to local consumers and residents.
21. Screen or Screening - To use landscape materials, walls, fencing, berms, or other material to shield an area from view of the public and/or to mitigate noise impacts.
22. Sight Distance Triangle - That area formed by drawing a straight line back from intersecting property lines 25 feet from said intersection and connecting same with a separate line, creating a triangle.
23. Signs - Any structure or part thereof or any device attached to a structure, or any other form of visual communication applied by paint, illumination, embossing or other
technique to a building or other structure for the purpose of directing, advertising, informing, warning or otherwise conveying information visually to the viewer.
24. Stormwater Detention - Containment of controlled runoff temporarily for storage before discharging downstream. Typically the water is stored in a pond for a limited period of time.
25. Structure - A combination of materials other than natural terrain or plant growth erected or constructed to form a shelter, enclosure, retainer, container, support, base, pavement or decoration.
26. Xeriscape - An environmentally friendly and water wise landscape design approach where some or all of the following techniques are utilized; 1) selecting low water demand plantings, 2) grouping plants by their specific needs, 3) reducing turf areas, 4) using turf types with low water requirements, 5) using plants native to the region being designed, 6 ) using mulches to cover soil and save moisture, 7) irrigating by zoning those plants together with similar water needs and by using efficient sprinkler head layout and water distribution patterns, 8) and performing regular maintenance to preserve the landscape and conserve water.

## EXHIBIT A

## Architectural Design Guidelines Standards Mountain View West - Residential and Multifamily Buildings


#### Abstract

The following information is intended for use as an outline only. Please refer to the recorded Covenants, Conditions and Restrictions for Mountain View West Subdivision for additional detail. The Covenants require that all items are to be submitted to the Architectural Review Committee ("ARC") for review and approval before submittal to the Town of Johnstown Review Committee ("JRC") for their approval. Both approvals are required prior to construction commencing on any residential or multifamily building.


| ITEM | GUIDELINE |
| :---: | :---: |
| Design Scheme | Neo-Traditional "Craftsman Style" |
| Roof Materials | Residential - Clay or concrete tile or Minimum 50 Year architectural asphalt shingle Commercial - May also add colored metal or flat roof with EPDM membrane |
| Roof Slopes/Overhang | Minimum 6/12 pitched roof. Sub roof structures may have less pitch to add character to the main roof. (i. e. dormers) Minimum 12 inch over hang on residential units. |
| Masonry | Brick, stone or faux stone materials wrapped a minimum 4 feet around sides on residential, multifamily and commercial facades. This shall include garages. |
| Siding | Cementous, Wood or Masonite lap siding is allowed. Maximum width allowed is 8 inches. Maximum exposure allowed is seven inches. Horizontal grooved is allowable. (Panelized siding such as T-111 is not allowed.) |
| Trim Widths |  |
| Following are required: | Minimum 1" by 4" width required for all windows (front, rear and sides) |
|  | Minimum $1^{\prime \prime}$ by 6 " width required for all corners |
|  | Minimum $1^{\prime \prime}$ by $10^{\prime \prime}$ width required at all floor changes and gable ends |
|  | Minimum $1^{\prime \prime}$ by $10^{\prime \prime}$ width required at bottom of siding above foundation |
| Fascia | $1^{\prime \prime}$ by $8^{\prime \prime}$ width with a $1^{\prime \prime}$ by $4 "$ trim or gutter. Fascia must be wood or CTX only. Seven sixteenth inch ( $7 / 16^{\prime \prime}$ ) siding shall not be allowed to be used as fascia. |
| Exterior Vents | Must fit exterior design and be colored to match adjacent materials |
| Vertical Support Posts | Must be framed with a minimum $8^{\prime \prime}$ width on the street facing side, minimum 6 " on the sides. Supports shall be wider at the bottom that at the top. Masonry or stone must be a minimum of $12^{\prime \prime}$ on any elevation when used. |
| Heat \& Plumbing Vents | Must be located on the roof slope away from the street elevation or screened and hidden. |
| Colors | Traditional colors to blend with the character of the neighborhood are allowed. All exterior railings, wood, trim, etc. shall match in color. Clear finish is not allowed. Adequate representation of proposed colors will be submitted to the ARC and JRC |
| Windows | All windows must be wood or vinyl. Aluminum windows are not allowed. The exterior of windows shall be painted to match trim colors. |
| Fences | See "Fencing Standards" within the MVW Covenants (CC\&Rs). |
| Landscaping | See "Landscape Plan" within the MVW Covenants (CC\&Rs). |
| Antennas \& Dishes Items Allowable With | Antennas are allowed only in attic spaces. Satellite dishes are allowed per Covenants. |
| Special Use Approval | Storm Doors, basketball hoops, playhouses, dog houses and dog runs, swing sets, signs, site lighting located off building structures, firewood storage, change in color scheme. |

## EXHIBIT A CONTINUED



## Craftsman Detail Options

Identifying characteristics and features include such things as pitched and occasionally hipped roofs with wide, and sometimes unenclosed overhangs, roof rafters (exposed) or architectural characteristics shown that represent the look of exposed rafters or decorative false beams or braces, commonly added under gables, porches, either full or partial width, with roof support columns many times tapered as shown on these representative drawings. On these pages of Exhibit B are representative photos of styles and characteristics of architectural features the Mountain View West Architectural Design Review Committee would like to see incorporated within your design submittal.


EXHIBIT A CONTINUED
Craftsman Detail Options


EXHIBIT A CONTINUED
Craftsman Detail Option


## Exhibit B

## MVW Commercial/Office \& Flex Space Design Guidelines

Craftsman Detail Options


## EXHIBIT C - MVW LANDSCAPE STANDARDS - XERISCAPE



## What is Xeriscaping?

Xeriscape, pronounced (ZER-i-scape), is a landscape practice used to promote water conservation through the design of creative and attractive water efficient landscapes. Xeriscaping is not the same as "zeroscaping" where the designed landscape consists mostly of hard surfaces with very few plants. Xeriscaping is also different from "natural/native" landscaping because here the emphasis is on the selection of plants for water conservation, not necessarily selecting native plants.

It is not a specific look or style. Rather, xeriscape is a combination of seven common-sense horticulture principles that save water, time and resources while creating a beautiful landscape.

## The Seven Principles of Xeriscape are:

1. Planning and Design: Whether you plan to design your own property or get help from a Landscape Architect/Designer. A plan is an important first step, a good design will provide direction and guidance to ensure that water-conserving techniques are coordinated and implemented in the landscape. Think about how you want to use your new Xeriscape, while considering maintenance.

Perform a site analysis of your property, take into account existing site features such as the location and orientation of your home (north, south, east or west), commercial building or other project feature, such as open space or entry feature, slopes, soils, drainage, downspouts, prevailing winds, sun exposure, activity areas, desirable views, privacy/screening needs, future structures and site improvements. Draw a base map of your property to scale (i.e., $1^{\prime \prime}=10^{\prime}-0^{\prime \prime}$ or $1 / 8^{\prime \prime}=1^{\prime}-0$ ", etc...) and begin to design your Xeriscape according to your future vision and needs.
2. Improve the Soil: A good soil, one that supports healthy plant life and conserves moisture is an important part of any healthy landscape. Before any planting, add organic matter such as compost or well-aged manure.

For most soils, adding 3-5 cubic yards of organic matter per 1,000 square feet of area to your soil can be beneficial for plant health, soil infiltration and water retention. Rototill the organic matter into the soil at a depth of at least 6 inches.
3. Appropriate Plant Selection: Choose plant species according to their sun and water requirements as it pertains to your specific site and areas within your property. Each property has its own set of criteria when it comes to sun exposure (sunny and shady areas) as well as drainage areas (dry or wet areas). Group plants of similar sun and water requirements together and place them in an area of the site which matches these requirements.

Provided with this document is a Plant List for guidance to get you started. Understand there are many plant varieties not provided on this list that could work in your Xeriscape, with approval. For additional plant options please reference the websites listed on page 4 of this document or visit local nurseries.
4. Practical Turf Areas: Thoughtful placement of turf areas of manageable size, shape and selection of appropriate drought tolerant turf species are a must. Consider limiting turf grass to high-traffic areas close to the house or other building, such as areas for play, recreation, and pets, with turf grasses that have been hybridized for arid conditions, such as Hybrid Bluegrass varieties and Turf-Type Tall Fescue. Native turf grasses such as Buffalo Grass or Blue Grama perform much better in low-traffic areas. Avoid narrow strips of turf grass which are hard to maintain and water. Consider planting landscape medians with low water, low maintenance plant material.
5. Irrigation: Establish hydrozones for water use. Group plant varieties and turf areas according to specific watering needs by dividing the Xeriscape into zones: High (regular watering), Moderate (occasional watering) and Low (little to no watering). Design an irrigation system to water appropriately and conserve water by zoning the irrigation system to serve plant groupings of similar water needs. This can be accomplished by irrigating turf areas separately (with a pop-up sprinkler system) apart from other planting bed/shrub areas (with low-volume drip irrigation). Irrigate areas according to their specific needs by applying the correct amount of water at the correct time of day, early morning or late evening.

Consider the design of your irrigation system at the same time as the design is being completed of your planting plan to minimize the potential for water waste.
6. Mulch Planting Beds: Mulch shall be shredded bark, bark chips, rock, and/or gravel.

Organic Mulch, such as shredded wood and bark chips, should be applied at a depth of at least 34 inches and will help keep plant roots cool, prevent water evaporation from the soil and will reduce weed growth. Keep in mind that Colorado winds tend to disperse dried out organic mulch.

Inorganic Mulch, such as rock and gravel should be applied at a depth of approximately 2" thick over a weed barrier fabric. Keep in mind extensive use of rock on south and west exposures can raise temperatures near the house, building or other structure and result in wasteful water runoff. Because of the heat that radiates from rock mulch, consider only hardy shrubs and trees to be planted in these conditions. Mulch will give planting beds a finished look and increase the visual appeal of your landscape.
7. Landscape Maintenance: Properly mowing, weeding, pruning, watering and fertilizing at the correct time will preserve the beauty of the Xeriscape. A well-maintained landscape will be healthier and hardier to better withstand drought. Once established, Xeriscape Landscapes, require less maintenance and less irrigation than Traditional Landscapes.

## MOUNTAIN VIEW WEST XERISCAPE PLAN REQUIREMENTS:



The Mountain View West Master \& Homeowners Associations encourage and support tasteful Xeriscaping which will not only beautify our neighborhood, but also lower outdoor water use up to 50 percent. Current Design Guidelines state that all Xeriscape plans require Committee approval.

Xeriscaping does not involve creating a hot dry landscape by dumping truckloads of rock and gravel on to your property. Only aesthetically pleasing Xeriscape plans will be approved by the Mountain View West HOA Architectural Board. Prior to submitting an Architectural Review Request for your Xeriscape, consider the following:

## Single Family Residential, Multi-Family Residential and Commercial Standards:

1. Plan for Submittal: Prior to installing the landscape of a property, the Owner must submit an ARC Approval Application. There must be an overall design which enhances the look of the home or other buildings and complies with the vision of the neighborhood. The request must include an outline of the project. Plan view designs must provide detailed information on the location of existing site features and all proposed site elements such as locations of hardscape, turf, mulch types and plant material drawn to their mature sizes. Plans must indicate location and types of mulch and rock. If detailed drawings are not included with the Architectural Review Committee Approval Application, the ARC Approval Application will be returned to the homeowner or building property owner.
2. Ground Cover: May include turf, native turf species, or perennial/shrub no-mow groundcovers. Wood mulch, rock mulch, decorative rock boulders, or other natural material over fabric to provide a neat, dust-free, weed-free appearance.

Large areas may not be composed of a single material, i.e. bare mulch/rock unless interspersed with groupings of plants.
3. Borders: Edging may consist of metal edging and masonry products such as concrete edger. Turf/native turf areas must be bordered to clearly define turf from planting beds.

## 4. Front Yard Standards:

a. The front yard must have a minimum of $30 \%$ and a maximum of $50 \%$ irrigated turf or alternative turf area, such as no-mow perennial groundcovers. Sprinkler controllers must be set to water turf and planting bed areas in compliance with the Town of Johnstown.
b. The front yard must have a maximum of $25 \%$ tastefully organized inorganic coverings such as rock, stone, or gravel (or some combination thereof).
c. The remaining area should be perennials, shrubs, trees, and organic mulched area.
d. Once installed, the landscaping must be maintained in a neat, attractive, and well-kept manner. Remove dead trees and shrubs promptly. Do not allow weeds to grow in the mulched or rock areas.
e. Turf grass must be watered sufficiently to prevent it from dying or going dormant while not exceeding water provider’s limitations.
5. Side Yard Standards:
a. In any location where the side yard of a corner lot is exposed to a street in front of a fence, the side yard landscaping shall be integrated with the front yard landscaping and subject to the same standards.
6. Back Yard Standards:
a. The same seven water saving principles are to be applied to backyard designs.
7. Committee Approval: Does not constitute assurance that landscape improvements comply with the Landscape Standards and Specifications of the Town of Johnstown. Property owners are responsible for all permits and approvals required from the Town of Johnstown.


City of Johnstown Landscape Standards:
http://www.townofjohnstown.com/DocumentCenter/Home/View/170

## Sources for further Xeriscape and Plant Selection information:

http://coloradowaterwise.org/page-645743
http://extension.colostate.edu/topic-areas/yard-garden/xeriscaping-creative-landscaping-7-228/
http://www.denverwater.org/Conservation/Xeriscape/XeriscapePlans/ http://www.fcgov.com/utilities/residential/conserve/water-efficiency/xeriscape http://www.highcountrygardens.com/

## EXHIBIT C CONTINUED - MVW LANDSCPAPE - PLANT LIST

## PLANT LIST

## BOTANICAL NAME <br> DECIDUOUS TREES

| Acer glabrum | Rocky Mountain Maple | L | PS-FS |
| :---: | :---: | :---: | :---: |
| Acer grandidentatum | Bigtooth Maple | VL-L | PS-FS |
| Acer tataricum | Tatarian Maple | L | PS-FS |
| Acer tataricum 'Garann' | Hot Wings Tatarian Maple | L | PS-FS |
| Aesculus glabra | Ohio Buckeye | M | PS-FS |
| Aesculus hippocastanum | Horsechestnut | M | PS-FS |
| Amelanchier x grandiflora | Autumn Brilliance Serviceberry | L | PS-FS |
| Amelanchier canadensis | Shadblow Serviceberry | L | PS-FS |
| Catalpa speciosa | Western Catalpa | L-M | FS |
| Chionanthus virginicus | White Fringe Tree | M | PS-FS |
| Crataegus spp. | Hawthorn | L | PS-FS |
| Celtis occidentalis | Hackberry | L | PS-FS |
| Gleditsia spp. | Honeylocust | L-M | PS-FS |
| Gymnocladus dioica | Kentucky Coffeetree | L | PS-FS |
| Koelreuteria paniculata | Golden Rain Tree | L | PS-FS |
| Malus spp. | Crabapple | M | PS-FS |
| Ptelea trifoliata | Wafer Ash (Hop Tree) | L-M | S-PS-SF |
| Pyrus spp. | Ornamental Pear | M | PS-FS |
| Quercus gambelli | Gambel Oak | VL-L | PS-FS |
| Quercus macrocarpa | Burr Oak | VL-L | FS |
| Quercus muehlenbergii | Chinkapin Oak | L-M | FS |
| Robina pseudoacacia 'Purple Robe' | Purple Robe Locust | VL-L | FS |
| Sophora japonica | Japanese Pagoda Tree | M | FS |
| Syringa reticulata | Japanese Tree Lilac | M | FS |
| Tilia spp. | Linden | M | PS-FS |
| Ulmus spp. | Elm | L-M | PS-FS |
| ${ }^{1}$ HYDROZONE: <br> VL -Very Low Water Use - indicates a plan <br> L - Low Water Use - indicates a plant that <br> M - Moderate Water Use - indicates a pla <br> H - High Water Use (None Selected) - in ${ }^{2}$ EXPOSURE: S-Shade PS-Part Sh | that requires little to no additional water o equires an additional 3 gallons of water pe that requires an additional 10 gallons of w cates a plant that requires continual water. | stablished are foot p per square | eason. |

## PLANT LIST

## BOTANICAL NAME

COMMON NAME
HYDROZONE ${ }^{1}$ EXPOSURE ${ }^{2}$

## EVERGREEN TREES

| Abies concolor | White Fir | L-M | PS-FS |
| :--- | :--- | :---: | :---: |
| Juniperus spp. | Juniper | L | PS-FS |
| Picea abies | Norway Spruce | M | PS-FS |
| Picea pungens | Colorado Spruce | L-M | PS-FS |
| Pinus aristata | Bristlecone Pine | L | FS |
| Pinus cembroides edulis | Pinyon Pine | L | PS-FS |
| Pinus flexilis | Limber Pine | L | FS |
| Pinus flexilis 'Vanderwolf Pyramid' | Vanderwolf's Pine | L | FS |
| Pinus heldrichi v. leucodermis | Bosnian Pine | M | FS |
| Pinus mugo 'Big Tuna' | Big Tuna Mugo Pine | L | FS |
| Pinus mugo 'Tannenbaum' | Tannenbaum Mugo Pine | L | FS |
| Pinus nigra | Austrian Pine | L-M | FS |
| Pinus ponderosa | Ponderosa Pine | L-M | FS |
| Pinus spp. 'character' | Character Pine | L | FS |

## DECIDUOUS SHRUBS

| Amelanchier spp. | Serviceberry | L | PS-FS |
| :--- | :--- | :---: | :---: |
| Amorpha spp. | Leadplant | L | FS |
| Aronis spp. | Chokeberry | L | FS |
| Artemisia spp. | Sage | VL-L | PS-FS |
| Atriplex spp. | Saltbush | VL-L | FS |
| Berberis spp. | Barberry | L | FS |
| Buddleia spp. | Butterfly Bush | L-M | FS |
| Caragana spp. | Peashrub | VL-L | PS-FS |
| Caryopteris spp. | Blue Mist, Dark Knight Spirea | L | FS |
| Cercocarpus spp. | Mahogany | VL | PS-FS |

## ${ }^{1}$ HYDROZONE:

VL -Very Low Water Use - indicates a plant that requires little to no additional water once established.
L - Low Water Use - indicates a plant that requires an additional 3 gallons of water per square foot per season.
M - Moderate Water Use - indicates a plant that requires an additional 10 gallons of water per square foot per season.
H - High Water Use (None Selected) - indicates a plant that requires continual water.
${ }^{2}$ EXPOSURE: S-Shade PS-Part Shade FS-Full Sun

PLANT LIST

| BOTANICAL NAME | COMMON NAME | HYDROZONE ${ }^{1}$ | EXPOSURE ${ }^{2}$ |
| :---: | :---: | :---: | :---: |
| Chamaebatiaria millefolium | Fernbush | VL-L | FS |
| Chrysothamnus spp. | Rabbitbrush | VL-L | FS |
| Cornus spp. | Dogwood | L-M | PS-FS |
| Cotoneaster spp. | Cotoneaster | L-M | S-PS |
| Cowania mexicana | Cliffrose | L | S-PS |
| Cytisus spp. | Broom | L | FS |
| Fallugia paradoxa | Apache Plume | VL-L | FS |
| Fendlera rupicola | Cliff Fendler Bush | L-M | FS |
| Forestiera neo-mexicana | New Mexican Privet | VL-L | FS |
| Holodiscus dumosus | Rock Spirea | L-M | PS-FS |
| James americana | Waxflower | VL-L | PS-FS |
| Kolkwitzia amabilis | Beauty Bush | L-M | PS-FS |
| Ligustrum spp. | Privet | L-M | PS-FS |
| Perovskia atriplicifolia | Russian Sage | L | PS-FS |
| Philadelphus spp. | Mockorange | L-M | PS-FS |
| Physocarpus spp. | Ninebark | L-M | PS-FS |
| Potentilla fruticosa spp. | Potentilla | L | PS-FS |
| Prunus besseyi spp. | Sandcherry | VL-L | PS-FS |
| Prunus tomentosa | Nanking Cherry | L | FS |
| Rhamnus | Buckthorn | L | PS-FS |
| Rhus glabra | Smooth Sumac | VL-L | PS-FS |
| Ribes spp. | Currant | L-M | S-PS-FS |
| Rosea spp. | Shrub Rose sp. | L-M | FS |
| Rubus deliciosus | Boulder Raspberry | VL-L | PS-FS |
| Shepherdia argentea | Silver Buffaloberry | VL-L | PS-FS |
| Sibiraea laevigata | Altai Spirea | L | PS-FS |
| Sibiraea altaiensis | Siberian Spirea | L | PS-FS |
| Sorbaria sorbifolia | Ash Leaf Spirea | L | PS-FS |
| ${ }^{1}$ HYDROZONE: |  |  |  |
| M - Moderate Water Use - indicates a plant that requires an additional 10 gallons of water per square foot per season. <br> H - High Water Use (None Selected) - indicates a plant that requires continual water. |  |  |  |
| ${ }^{2}$ EXPOSURE: $\quad$ S-Shade PS | FS-Full Sun |  |  |

## PLANT LIST

| BOTANICAL NAME | COMMON NAME | HYDROZONE ${ }^{1}$ | EXPOSURE $^{2}$ |
| :--- | :--- | :---: | :---: |
| Symphoricarpos spp. | Snowberry, Coralberry | L |  |
| Syringa spp. | Lilac | $\mathrm{L}-\mathrm{M}$ | PS-FS |
| Viburnum spp. | Viburnum | $\mathrm{L}-\mathrm{M}$ | PS-FS |

## EVERGREEN SHRUBS

| Juniperus spp. | Dwarf Juniper | L-M | PS-FS |
| :--- | :--- | :---: | :---: |
| Picea spp. | Dwarf Spruce | L | PS-FS |
| Pinus spp. | Dwarf Pine | L | PS-FS |

## BROADLEAF EVERGREENS

Agave parryi
Agave neomexicana
Arctostaphylos spp.
Euonymus fortunei 'Coloratus'
Euonymus fortunei 'Emerald Gaiety'
Euonymus fortunei 'Emerald n' Gold'
Hesperaloe parviflora
Mahonia aquifolium spp.
Yucca baccata
Yucca glauca

| Agave | VL-L | FS |
| :--- | :---: | :---: |
| New Mexico Agave | VL-L | FS |
| Manzanita | L-M | S-PS |
| Wintercreeper | L-M | S-PS |
| Emerald Gaiety Euonymus | L-M | S-PS |
| Emerald 'n Gold Euonymus | L-M | S-PS |
| Red Yucca | VL-L | FS |
| Oregon Grape Holly | VL-L | S-PS |
| Banana Yucca | VL-L | FS |
| Soapweed Yucca | VL-L | FS |

## ORNAMENTAL GRASSES

| Andropogon gerardii | Big Bluestem | L-M | FS |
| :---: | :---: | :---: | :---: |
| Bouteloua gracilis 'Blonde Ambition' | Blonde Ambition Blue Grama | VL-L | PS-FS |
| Calamagrostis spp. | Reed Grass | VL-L | PS-FS |
| Erianthus ravennae | Hardy Pampas Grass | VL-L | PS-FS |
| Festuca spp. | Blue Fescue | VL-L | PS-FS |
| Hakonecbola spp. | Japanese Forest Grass | VL-L | S-PS |
| Helictotrichon sempervirens | Blue Avena Grass | L | PS-FS |
| ${ }^{1}$ HYDROZONE: <br> VL -Very Low Water Use - indicates a plan <br> L - Low Water Use - indicates a plant that <br> M - Moderate Water Use - indicates a plan <br> H - High Water Use (None Selected) - indi | hat requires little to no additional water equires an additional 3 gallons of water p that requires an additional 10 gallons of ates a plant that requires continual wate | stablishe are foot er squar |  |
| ${ }^{2}$ EXPOSURE: $\quad$ S-Shade PS-Part Sha | , |  |  |

## PLANT LIST

| BOTANICAL NAME | COMMON NAME |  | HYDROZONE $^{1}$ | EXPOSURE $^{2}$ |
| :--- | :--- | :---: | :---: | :---: |
| Miscanthus purpurascens | Maiden Grass |  |  |  |
| Muhlenbergia spp. | Muhly Grass | L-M | PS-FS |  |
| Pancium virgatum spp. | Switchgrass | VL-L | FS |  |
| Pennisetum alopecuroides | Dwarf Fountain Grass | VL-L | FS |  |
|  |  | L-M | FS |  |

PERENNIALS

| Achillea spp. | Yarrow | VL-L | PS-FS |
| :---: | :---: | :---: | :---: |
| Agastache spp. | Agastache | L-M | FS |
| Alcea rosea | Hollyhock | L | PS-FS |
| Alyssum spp. | Alyssum | L | PS-FS |
| Antennaria parvifolia | Dwarf Pussytoes | VL-L | PS-FS |
| Aquilegia spp. | Columbine | L-M | PS-FS |
| Arctostaphylos uva-ursi | Kinnikinnick | L | FS |
| Artemisia spp. | Artemisia | VL | FS |
| Asclepias tuberosa | Orange Butterfly Weed | VL | FS |
| Aubrieta spp. | Rockcress | L | FS |
| Aurinia saxitile compactum | Basket of Gold | L-M | FS |
| Baptisa australis | False Indigo | L-M | FS |
| Berlandiera lyrata | Chocolate Flower | VL-L | FS |
| Callirhoe involucrata | Poppy Mallow | L-M | FS |
| Campanula spp. | Harebell | L | PS-FS |
| Centranthus ruber | Red Valerian | L-M | PS-FS |
| Ceratostigma plumbaginoides | Plumbago | L-M | PS-FS |
| Coreopsis spp. | Coreopsis | L | FS |
| Delosperma spp. | Ice Plant | L-M | FS |
| Diascia integerrima 'Coral Canyon' | Coral Canyon Twinspur | M | FS |
| Digitalis thapsi 'Spanish Peaks' | Dwarf Pink Foxglove | L-M | PS-FS |
| Duchesnea indica | Mock Strawberry | L-M | PS-FS |
| Echinacea spp. | Coneflower | M | FS |
| ${ }^{1}$ HYDROZONE: <br> VL -Very Low Water Use - indicates a plant <br> L - Low Water Use - indicates a plant that re <br> M - Moderate Water Use - indicates a plant <br> H - High Water Use (None Selected) - indic ${ }^{2}$ EXPOSURE: S-Shade PS-Part Shade | that requires little to no additiona equires an additional 3 gallons of that requires an additional 10 ga ates a plant that requires continu FS-Full Sun | stablished are foot p per square |  |

## PLANT LIST



## PLANT LIST

| BOTANICAL NAME | COMMON NAME | HYDROZONE ${ }^{1}$ | EXPOSURE ${ }^{2}$ |
| :---: | :---: | :---: | :---: |
| Pulsatilla vulgaris | Pasqueflower | L | PS-FS |
| Ratibida columnifera 'Mexican Hat' | Prairie Coneflower | VL | FS |
| Rudbeckia spp. | Gloriosa Daisy | L | FS |
| Salvia spp. | Salvia | L-M | FS |
| Saponaria ocymoides 'Splendens' | Rock Soapwort | L | FS |
| Santolina chamaecyparissus | Lavender Cotton | L-M | FS |
| Scabiosa spp. | Pincushion Flower | L | PS-FS |
| Scutellaria resinosa | Prairie Skullcap | L-M | FS |
| Sedum spp. | Sedum | L | PS-FS |
| Sempervirum spp. | Hens and Chicks | VL-L | PS-FS |
| Sphaeralcea spp. | Orange Globemallow | VL-L | FS |
| Stachys byzantina 'Silver Carpet' | Silver Carpet Lamb's Ear | L | PS-FS |
| Tanacetum spp. | Partridge Feather | VL | FS |
| Teucrium spp. | Germander | L-M | FS |
| Thymus spp. | Thyme | L | PS-FS |
| Verbena bipinnatifida | Native Verbena | L | PS-FS |
| Veronica spp. | Veronica | L | PS-FS |
| Viola corsica | Corsican Violet | VL | PS-FS |
| Zauschneria spp. | Hummingbird Trumpet Flower | L | FS |
| Zinnia grandiflora | Rocky Mountain Zinnia | VL-L | FS |
| VINES |  |  |  |
| Campsis radicals | Trumpet Vine | L-M | PS-FS |
| Clematis terniflora | Autumn Clematis | L-M | PS-FS |
| Lonicera spp. | Honeysuckle Vine | L-M | PS-FS |
| Polygonum aubertii | Silverlace Vine | L-M | PS-FS |
| VL -Very Low Water Use - indicates a plant that requires little to no additional water once established. <br> L - Low Water Use - indicates a plant that requires an additional 3 gallons of water per square foot per season. <br> M - Moderate Water Use - indicates a plant that requires an additional 10 gallons of water per square foot per season. <br> H - High Water Use (None Selected) - indicates a plant that requires continual water. |  |  |  |
| ${ }^{2}$ EXPOSURE: $\quad$ S-Shade PS-Part Sha | FS-Full Sun |  |  |

## BOTANICAL NAME

COMMON NAME
HYDROZONE ${ }^{1}$ EXPOSURE ${ }^{2}$

## TURF

| Agropyron spp. |  |  |  |
| :--- | :--- | :---: | :---: |
| Agropyron cristatum | Ephraim Crested Wheatgrass | L | FS |
| Buchloe dactyloides | Buffalo Grass | VL-L | FS |
| Bouteloua gracilis | Blue Grama | L | FS |
| Bromus interims | Smooth Bromegrass | L | FS |
| Festuca arundinacea spp. | Turf-Type Tall Fescue | M | PS-FS |
| Festuca spp. | Fine Fescues | L-M | S-PS |
| Poa pratensis x Poa arachnifera | Texas Hybrid, Bandera/ | M | FS |
|  | Texas Hybrid, SPF30 Blend |  |  |

[^27]The Johnstown Review Committee (JRC) has adopted these Mountain View West (MVW) Design Guidelines as of this day of 2017 by Town Council Resolution.

## Town of Johnstown

Settler's Crossing

Preliminary/Final Subdivision Plat (SUB23-0003)

> Planning and Zoning Commission Meeting November 1, 2023

- General Location: East of Parish Ave \& South of Settler's Way
- Approx. 13.7 acres
- Previous Planning \& Zoning Events
- 2005- Johnstown Colony Annexation
- 2017-Mountain View West PUD



## Development \&e Subdivsion

- 11 commercial lots
- Access Easement has been approved for lots 10 \& 8 from CR 46.5
- Part of the Mountain View West Development

- Conforms to the Mountain View West Land Use Plan along with the Town's codes \& compliances
- Recommendation for Approval


# Planning \& Zoning Commission Staff Analysis Report 

ITEM:

## PROJECT:

PARCEL NO:
DESCRPTION:

LOCATION:
DEVELOPER:
STAFF:

HEARING DATE:

Public Hearing and Consideration of the Blue-Sky Prairie Subdivision Preliminary Plat

SUB23-0008
106102100021 \& 106102100031
Residential Subdivision of approximately 121 acres, in the recently Amended High Plains Village PD

South of Veteran's Parkway \& East of High Plains Blvd
High Plains Estates JV, LLC
Lilly Cory, Planner I

November 1, 2023

## ATTACHMENTS:

1. Preliminary Plat
2. High Plains Village PD
3. Traffic Study Plan

## EXECUTIVE SUMMARY:

The Developer, High Plains Estates JV, LLC, associated with Henry Design Group \& LJA Engineering is requesting approval of the Preliminary Plat/Subdivision within the High Plains Village PD. The subdivision will consist of $\sim 425$ lots, dedicated right-of-way ROW, and connective out lots for drainage and open space. The internal collector road - Waterbirch Avewill have a more heavily landscaped ROW to create a more open walkway in the community to make up for 0.6 acres of the open space requirement as per the land use code. The subdivision will outline the general overview of the expected development and use of the land.

ZONING: PD- High Plains Village PD Amendment 1 (Attachment 2)

## ADJACENT ZONING \& LAND USE:

North: Great Plains Village PD- Revere North Development, Residential
East: PD- Cito Property, Agriculture
South: R1- Rocksbury Ridge
West: Great Plains Village PD- Revere Development, Residential

## PROPERTY LAND USE HISTORY

The subject property was historically used for farming and was annexed into Johnstown as part of the Klein 125 Annexation in 2008 by Ordinance \#2004-720, reception \#20043190851: PD High Plains Village by ordinance \#2004-721, reception \#20043190852. The subject property is currently unplatted. The High Plains Village PD was Amended in 203 by ordinance \#2023-241, reception \#20234902691(Attachment 2).

## SUBMITTAL AND REFERRALS

This project submittal included the proposed preliminary subdivision plat, as well as required engineering plans and reports for the site. The project was referred to and reviewed by:

| + Weld County | + Helton \& Williamsen (Water Engineer) |
| :--- | :--- |
| + IMEG (Town Engineer) | + NOCO Water Conservation District |
| + Little Thompson Water District | + Public Works \& Utilities |
| + Front Range Fire District | + FHU (Town Traffic Engineer) |

## PROJECT DESCRIPITON \& ANALYSIS

The proposed subdivision would create 3 new Filings in the High Plains Village PD. This would outline 425 lots and 32 outlots for open space. The proposed plat will connect to High Plains Boulevard and Veteran's Parkway (WCR 50) with one entrance/exit on each road.

All outlets will be used in open space, spacing from oil wells (plugged \& abandoned), or drainage (detention ponds) for the development. At this time a Water Sewar Service Agreement or a Development Agreement will not be required as no official lots will be made. When the official Final Development \& Subdivision is to go through council this will occur.

At this time there are no public improvements that have been proposed. There is a reimbursement agreement with Forestar (Revere Development) for the expansion of High Plains Blvd.

The staff has no outstanding concerns with this subdivision and believes that it will promote the Town's housing diversity and local economy. There are no concerns of incompatibility with surrounding Town Developments \& Zoning.

## The Community That Cares

johnstown.colorado.gov

## PUBLIC NOTICE

Notice for the Planning and Zoning Meeting and Public Hearing was published in the Johnstown Breeze on Thursday September 26, 2023. This notice provided the date, time, and location of the hearing along with a description of the project. Notices were mailed out on Tuesday September 24,2023 to all landowners in an 800 ft radius from the property. The meeting for October $18^{\text {th }}$ was cancelled so all notifications for the landowners and the Johnstown Breeze were sent out on October 19 ${ }^{\text {th }}, 2023$.

## RECOMMENDED FINDINGS AND MOTIONS

## Recommended Findings

It is recommended that the Planning and Zoning Commission send a recommendation for approval to Town Council for the Preliminary Plat for Blue-Sky Prairie based upon the following findings:
1.) The proposed subdivision is consistent with the town comprehensive plan and is in compliance with the Town's codes, regulations, and requirements along with the High Plains Village PD.
2.) The proposed subdivision meets the updated requirements of a PD by creating a unique neighborhood by using their landscaping and lot diversity.

## Recommended Motion to Approve

I move to approve Blue Sky Prairie Subdivision Preliminary Plat/Development Plan, based on the application received, information provided, and findings noted at this hearing.

## Alternative Motions:

## Motion to Recommend Approval with Conditions

I move to approve Blue Sky Prairie Subdivision Preliminary Plat/Development Plan with conditions, based on the application received, information provided, and findings noted at this hearing. The conditions are listed as such:

## Motion to Recommend Denial

I move that the Commission recommend to Town Council Denial of the Blue Sky Prairie Subdivision Preliminary Plat/Development Plan, based on the following findings.

## BLUE SKY PRAIRIE

A PARCEL OF LAND BEING A PORTION OF THE NORTHEAST QUARTER OF SECTION 2 TOWNSHIP 4 NORTH, RANGE 68 WEST OF THE SIXTH PRINCIPAL MERIDIAN TOWN OF JOHNSTOWN, COUNTY OF WELD, STATE OF COLORADO.

## LEGAL DESCRIPTION

 ano


Contaning an area of 121.160 Acres. (5.277.732 souare feet), more or less.

## general notes




 MANAEMENT AGENC
DCCEMER 19 , 2006 .








VICINITY MAP


## APPROVALS

## PLANnNa and zonne comussion


BY: CHARR P PANNIG ANO ZONNG Comvisson

## own councli




BY: $\quad$ MAYOR ATEST: $\frac{\text { TOWN CLERK }}{\text { M }}$

## Tow enaner

Town Encnerer, Town of Jonnsion

## owner/APPLICANT

## PLANNER

## 

## ENGEER






## SURVEYOR

| LAND SUMMARY CHART |  |  |  |
| :---: | :---: | :---: | :---: |
|  |  | Acres |  |
| Trs (413 Total) | 2.072 .213 | 47.6 |  |
| Of-way | 108504 |  |  |
| oulotis (32 total) | 2.114,47 | ${ }^{48.54}$ |  |
| totals | 5.27, |  |  |


| $\triangle \mathrm{AZTEC}$ |  |  | DEVELOPER <br> Cola, LlC/VIEW HoMES, INC <br>  |  |  | 20203 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | scate |  |  |
| O | Drame |  |  | shemt 1 of |  |  |



## BLUE SKY PRAIRIE

A PARCEL OF LAND BEING A PORTION OF THE NORTHEAST QUARTER OF SECTION 2


TOWNSHIP 4 NORTH, RANGE 68 WEST OF THE SIXTH PRINCIPAL MERIDIAN,


VICINITY MAP

## DEVELOPMENT PHASING

 Condtions ictat

APPROVALS
OWNERSHIP CERTIFICATION

By: View Hanes. Inconporated, a Texas coroporation

STATEOF COLORADO ; ss. $_{\text {s. }}$,
 Miness my hand and official seal - My commision oxprics 10 - $17-2026$
Sandraddarecta


ATTORNEY CERTIFICATION



JANESE FREDMAN

TOWN COUNCIL





| DEVELOPER | ENGINEER | PLANNER | SURVEYOR |
| :---: | :---: | :---: | :---: |
| JSCHA <br> COLA, LLC / VIEW HOMES <br> 719-306-2976 | KEVIN LOVELACE LJA ENGINEERING, INC 1765 WEST 121ST AVENUE SUITE 300 WESTMINSTER, CO 80234 $303-421-4224$ 303-421-4224 | KAREN HENRY HENRY DESIGN GROUP UNIT 1-C DENVER, CO 80202 $303-446-2368$ -303-446-2368 | JIM LYNCH 300 EAST MINERAL AVENU SUITE 1 LITTLETON, CO 80122 303-713-1898 303-713-1898 |

HIGH PLAINS ESTATES OUTLINE DEVELOPMENT PLAN
AMENDMENT NO. 1 TO KLEIN 125 ANNEXATION OUTLINE DEVELOPMENT PLAN
A PARCEL LOCATED IN THE NORTHEAST QUARTER OF SECTION 2
TOWNSHIP 4 NORTH, RANGE 68 WEST OF THE SIXTH PRINCIPAL MERIDIAN,
COUNTY OF WELD, STATE OF COLORADO

MONUMENT SYMBOL LEGEND


- founo monument as describe
- Found secton corner as described
(M) MEASured distance


## Statement of Intent



B. $\frac{\text { Neiatborhood Conceop }}{\text { The tigh Plans }}$









Features of the PUD inculude

1. An interconnected
ublic




Ceneral Communty Framevork



D. Site Desidn Digails

 | Eanshnus |
| :--- |
| Estates. |







2. General Notes

B. $\frac{\text { Conficics }}{\text { The provis }}$

 Johnstown Municiona
Districit shal apply,



D. $\frac{\text { Densiliv Ranges }}{\text { Residential densis }}$ permited number of dweling units and density shall ine feturtear speecified at

desisn and dinal designg due to enginee.ing findings. There is no minimum
density required in the Plañing
A teas.
$\frac{\text { Residential } 1 \text { Density Transer }}{\text { Transer of residential }}$






$\frac{\text { Planning } A \text { rea }}{\text { Planandaries }}$




## NOTE:

```
CONCEPTUAL PLANNING ELEMENTS WITHIN THIS ODP ARE SUBJECT TO FURTHER
DETAILED REVIEW AND UPDATED REQUREMENTS OF THE TOWN WITH
SUBIEQUNT SUBMITTAS FOR SUBEQUENT SUBMITTALS FOR
DEVELOPMENT AND CONSTRUCTION PLANS THESE ELEMENTS ARE INCLUDED TO
ILUUSTRATE FEASIBIITY OF THE SITE ILLUSTRATE FEASIBLITY OF THE SITE FOR
THE PROPOSED LEVEL OF DEVELOPMENT THE PROPOSED LEVEL OF DEVELOPMENT
AND PRESENT HIGH LEVEL DEVELOPMENT CONCEPTS ONLY
```



## LAND USE SUMMARY

| Plannocarea | ${ }_{\text {ACREGEE }}$ | densimpance | Maxmmmoweung unis | bforal |
| :---: | :---: | :---: | :---: | :---: |
| 1 |  | 3-120UGC |  |  |
|  | ${ }^{22.1}$ | ${ }_{\text {3-50VAC }}$ | ${ }_{\substack{131}}^{1}$ |  |
| $\begin{array}{r}3 \\ \hline\end{array}$ | ${ }_{\substack{283 \\ 30.2}}$ |  | ${ }_{\text {¢ }}^{172}$ |  |
| tens prwv |  |  | N/A |  |
| Total | 121.1 | N/A | 725 | 100\% |


| DEVELOPER | ENGINEER | PLANNER | SURVEYOR |
| :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { TIM BUSCHAR } \\ & \text { COLA, LLC / VIEW HOMES } \\ & 719-306-2976 \end{aligned}$ | KEVIN LOVELACE <br> LJA ENGINEERING, INC 1765 WEST 121ST AVENUE SUITE 300 WESTMINSTER, CO 80234 | KAREN HENRY HENRY DESIGN GROUP UNIT 1-C <br> DENVER, CO 80202 | JIM LYNCH <br> AZTEC CONSULTANTS 300 EAST MNERAL AVEN SuITE1 LTTLETON Co 80122 |

## OPEN AREA, PARKS, AND PEDESTRIAN CONNECTVITY

open area





 TrAIl Locations whe
PLAN AND FNALL PLAT.

## LEGEND

<-* 6'-8' PEDESTRIAN TRAIL

## Den DETACHED SIDEWALK

PUBLIC OR PRIVATE PARK / AMENITY
PRIMARY MONUMEN
SECONDARY MONUMENT
$\qquad$ LANDSCAPE BUFFER / OPEN SPACE
$\square$ detention


NOTE: CONCEPTUAL PLANNING ELEMENTS WITHIN THIS ODP ARE SUBJECT TO FURTHER
DTEALLEE REVIEW AND UPDATED DETAILED REVIEW AND UPDATED
REQUIREMENTS OF THE TOWN WITH SUBSEQUENT SUBMITTALSFOR DEVELOPMENT AND CONSTRUCTION PLANS ILUSTRATE FEASIBLITTY OF THE TITE THE PROPOSED LEVELOF DEVELOPMENT AND PRESENT HIGH LEVEL DEVELOPMENT
CONCEPTS ONLY.
$\frac{\text { DEVELOPER }}{\substack{\text { TMMUSCHAR } \\ \text { CTLA LLINVEWHOMES }}}$

PLANNER



 MENT ENSIN GROU

303.46-2368

## 

HIGH PLAINS ESTATES OUTLINE DEVELOPMENT PLAN
AMENDMENT NO. 1 TO KLEIN 125 ANNEXATION OUTLINE DEVELOPMENT PLAN

$$
\begin{aligned}
& \text { A PARCEL LOCATED IN THE NORTHEAST QUARTER OF SECTION } 2 \text {, } \\
& \text { rownship } 4 \text { NORTH, RANGE } 68 \text { WEST OF THE SIXTH PRINCIPAL MERIDIAI }
\end{aligned}
$$

COUNTY OF WELD, STATE OF COLORADO
ODP DEVELOPMENT STANDARDS AND DESIGN GUIDELINES
development standards notes
A. Permited Land Uses








3.



5.








Any housing type forao ower density may be developed within a higher
densily
2. AAley loaded singie efmily residential may front on a collector or a aterial
$\frac{\text { DESIIN GUIDELINE }}{1 .} \frac{\text { Apolicabily }}{}$

2. Acchitecture





3.





4. Buffering



5. Streetccapes









 Sight isisance ines:
distancone tinangines.
6. Fencing

Sionage
The intent of the signage within High plains EStates is to create u unifies sign



8. Lighting






| DEVELOPER | ENGINEER | PLANNER | SURVEYOR |
| :---: | :---: | :---: | :---: |
| TIM BUSCHAR <br> COLA, LIEW HOMES | KEVIN LOVELACE 1765 WEST 121 ST AVENUE <br>  WESTMINTTER, CO 80234 | KAREN HENRY <br> IGN GROUP <br> 1501 WAZEE ST <br> DENVER CO 80202 | JM LYYCH 300 EAST MNERAL AVENUE SuITE 1 |



$50^{\prime}$ SINGLE FAMILY DETACHED
FRONT LOADED 60' LOT TYPE


40' SINGLE FAMILY DETACHED
FRONT LOADED 50' LOT TYPE

$30^{\prime}$ SINGLE FAMILY DETACHED FRONT LOADED 40' LOT TYPE

$\int \frac{\text { SINGLE FAMILY DETACHED 'NOOK' }}{\text { ALLEY LOADED 35' LOT TYPE }}$


NOTE: CONCEPTUAL PLANNING ELEMENTS WITHIN THIS ODP ARE SUBJECT TOF FURTHER
DETALLED REVEW AND UPDATED DETALLED REVIEW AND UPDATED
REQUIREMENTS OF THE TOWN WITH REQUIREMENTS OF THE TOWN WITH
SUBSEQUENT SUBMITALS FOR SUBSEQUENT SUBMITALS FOR
DEVEOPMENT AND CONSTRCTON PLANS.
THESE ELEMENTS ARE INCUDD DEVELOPMENT AND CONSTRUCTION PLA
THESE EELEMENS ARINCUDE TO
ILUSTRATE FEASIBLITY OF THE SITE FOR ILESTRETENEASARLITY OF THE STIE FOR
IHUTRAPOSED LEVEL OF DEVELOPMENT
THE PRPSENT AND PRESENT HIGH LEVEL DEVELOPMENT
CONCEPTS ONLY.


$\bigcirc \frac{\text { PAIRED HOME }}{\text { FRONT LOADED }}$
FRONT LOADED
$\bigcirc \frac{\text { TOWNHOME- SINGLE FAMILY ATTACHED }}{\text { ALLEY LOADED FOUR PLEX }}$


PAIRED HOME
$\frac{\text { PAIRED HOME }}{\text { ALLEY LOADED }}$

$\frac{\text { TOWNHOME- SINGLE FAMILY ATTACHED }}{\text { FRONT LOADED FOUR PLEX }}$


-


NOTE:

| DEVELoper | ENGINEER | PLANNER | SURVEYOR |
| :---: | :---: | :---: | :---: |
|  | kevin lovelace LJA ENGINEERING, INC. 1765 WEST 121 IST AVENUE SUITE 300 WEST-411-4224, CO 80234 | karen henky <br>  UNTT - -C <br> $\underset{\substack{\text { DEEVER } \text { Co } \\ \text { 3030202 }}}{ }$ <br> 303-446-2368 | JIM LYNCH 300 EAST MNERAL AVENU sulte: 3037710 303-713-1898 |





# TRAFFIC IMPACT STUDY 

For

High Plains Estates<br>Weld County, Colorado



July 2022
Revised:
December 2022

Prepared for:
COLA, LLC
555 Middle Creek Parkway, Suite 500
Colorado Springs, CO 80921

Prepared by:
SM ROCHA, LLC
TRAFFIC AND TRANSPORTATION CONSULTANTS

8703 Yates Drive, Suite 210
Westminster, Colorado 80031
(303) 458-9798

6 South Tejon Street, Suite 515
Colorado Springs, Colorado 80903
(719) 203-6639

Project Engineer: Stephen Simon, EIT

Engineer in Responsible Charge: Fred Lantz, PE

## Table of Contents

I. Introduction ..... 1
Project Overview ..... 1
Study Area ..... 1
Site Description ..... 1
Existing and Committed Surface Transportation Network ..... 4
II. Existing Traffic Conditions ..... 6
Peak Hour Intersection Levels of Service - Existing Traffic ..... 8
Existing Traffic Analysis Results ..... 8
III. Future Traffic Conditions Without Proposed Development. ..... 10
Background Traffic Signal Warrant ..... 10
Peak Hour Intersection Levels of Service - Background Traffic ..... 14
Background Traffic Analysis Results - Year 2024 ..... 14
Background Traffic Analysis Results - Year 2042 ..... 15
IV. Proposed Project Traffic ..... 17
Trip Generation ..... 17
Adjustments to Trip Generation Rates ..... 18
Trip Distribution ..... 18
Trip Assignment ..... 18
V. Future Traffic Conditions With Proposed Developments ..... 20
VI. Project Impacts ..... 23
Peak Hour Intersection Levels of Service - Total Traffic ..... 23
Total Traffic Analysis Results Upon Development Build-Out ..... 24
Auxiliary Lane Analysis ..... 25
VII. Conclusion ..... 26

## List of Figures

Figure 1 - Location ..... 2
Figure 2 - Site Plan ..... 3
Figure 3 - Existing Traffic Volumes \& Intersection Geometry ..... 7
Figure 4 - Background Traffic Volumes \& Intersection Geometry - Year 2024 ..... 12
Figure 5 - Background Traffic Volumes \& Intersection Geometry - Year 2042 ..... 13
Figure 6 - Distribution and Site-Generated Assignment ..... 19
Figure 7 - Total Traffic Volumes \& Intersection Geometry - Year 2024 ..... 21
Figure 8 - Total Traffic Volumes \& Intersection Geometry - Year 2042 ..... 22
List of Tables
Table 1 - Intersection Capacity Analysis Summary - Existing Traffic ..... 8
Table 2 - Intersection Capacity Analysis Summary - Background Traffic - Year 2024 ..... 14
Table 3 - Intersection Capacity Analysis Summary - Background Traffic - Year 2042 ..... 15
Table 4 - Trip Generation Rates ..... 17
Table 5 - Trip Generation Summary ..... 17
Table 6 - Intersection Capacity Analysis Summary - Total Traffic - Year 2024 ..... 23
Table 7 - Intersection Capacity Analysis Summary - Total Traffic - Year 2042 ..... 24
Appendices
APPENDIX A TRAFFIC COUNT DATAAPPENDIX B LEVEL OF SERVICE DEFINITIONSAPPENDIX C CAPACITY WORKSHEETSAPPENDIXD WARRANT ANALYSIS FORMS

## I. Introduction

## Project Overview

This traffic impact study is provided as a planning document and addresses the capacity, geometric, and control requirements associated with the development entitled High Plains Estates.

This traffic impact study has been revised to address jurisdiction review comments regarding additional description and detail of proposed site access connection to River Rock Drive.

This proposed development consists of a residential subdivision including a mix of single-family and multifamily housing. The development is located on the south side of E County Road 14 and east of High Plains Boulevard in Weld County, Colorado.

## Study Area

The study area to be examined in this analysis encompasses High Plains Boulevard between E County Road 14 and State Highway 60, and E Count Road 14 from I-25 Frontage Road to High Plains Boulevard, as well as proposed site access drives.

Figure 1 illustrates location of the site and study intersections.

## Site Description

Land for the development is currently vacant and surrounded by a mix of residential, agricultural, and open space land uses. The proposed development is understood to entail the new construction of a residential subdivision supporting as many as 226 multifamily dwelling units, and 401 single-family detached dwelling units.

Proposed access to the development is primarily provided at the following locations: one fullmovement access onto High Plains Boulevard (referred to as Access A), and one full-movement access onto E County Road 14 (referred to as Access B). Additional access to the development area also includes connection to Onyx Place via extension of River Rock Drive, as well as future connection to the east. However, for analysis purposes said access locations were not directly analyzed given their internal nature to the overall residential development area and the conceptual nature of the site plan. Internal access operations are expected to provide levels of service equal to or better than those of the adjacent major study intersections.

For purposes of this study, it is anticipated that development construction would be completed by end of Year 2024. General site and access locations are shown on Figure 1. A conceptual site plan, as prepared by Henry Design Group, is shown on Figure 2. This plan is provided for illustrative purposes only.



## Existing and Committed Surface Transportation Network

Within the study area, High Plains Boulevard is the primary roadway that will accommodate traffic to and from the proposed development. The secondary roadways include E County Road 14, State Highway 60, Brunner Boulevard, l-25 Frontage Road, and River Rock Drive.

High Plains Boulevard is a north-south arterial roadway having between two to four through lanes (one to two lanes in each direction) with a combination of shared and exclusive turn lanes at the intersections within the study area. High Plains Boulevard provides a posted speed limit of 35 MPH . High Plains Boulevard currently ends north of Brunner Boulevard and is anticipated to be extended north to intersect E County Road 14 upon continued area development.

E County Road 14 is an east-west arterial roadway having two through lanes (one lane in each direction) with shared turn lanes at the intersections within the study area. E County Road 14 provides a posted speed limit of 55 MPH. E County Road 14 becomes State Highway 60 west of I-25 Frontage Road.

State Highway 60 is an east-west state roadway having two through lanes (one lane in each direction) with exclusive turn lanes at the intersection within the study area. The Colorado Department of Transportation (CDOT) categorizes State Highway 60 as a Non-Rural Principal Highway (NR-A) and provides a posted speed limit of 55 MPH.

Brunner Boulevard is an east-west collector roadway having two through lanes (one lane in each direction) with shared turn lanes at the intersection within the study area. Brunner Boulevard provides a posted speed limit of 25 MPH .

I-25 Frontage Road is a north-south state roadway having two through lanes (one lane in each direction) with shared turn lanes at the intersection within the study area. I-25 Frontage Road provides a posted speed limit of 55 MPH . I-25 Frontage Road is currently closed between E County Road 14 and State Highway 60 pursuant to CDOT's I-25 North Express Lanes Berthoud to Johnstown Project and is not anticipated to reopen in the future.

River Rock Drive is a north-south local roadway having two through lanes (one lane in each direction) with shared turn lanes at intersections within the existing adjacent subdivision. River Rock Drive does not provide a posted speed limit. However, based on the roadway classification, it is assumed to provide a posted speed limit of 25 MPH .

Study intersections operate under a stop-controlled condition. A stop-controlled intersection is defined as a roadway intersection where vehicle rights-of-way are controlled by one or more "STOP" signs.

No regional or specific improvements for the above-described roadways beyond that already discussed are known to be planned or committed at this time. It is anticipated that as area development continues to occur roadway and intersection improvements will be necessary dependent on specific capacity thresholds associated with each development area. For purposes of this analysis, the study area roadways are considered to be built to their ultimate cross-sections excluding future improvements associated with the proposed development and to accommodate regional transportation demands.

## II. Existing Traffic Conditions

Morning (AM) and afternoon (PM) peak hour traffic counts were collected at the intersections of High Plains Boulevard with State Highway 60 and Brunner Boulevard, as well as E County Road 14 with I25 Frontage Road. Average daily traffic (ADT) volumes were collected over a 24 -hour period on High Plains Boulevard, State Highway 60, and E County Road 14. Counts were collected on June 1, 2022, with AM peak hour counts being collected during the period of 7:00 a.m. to 9:00 a.m. and PM peak hour counts being collected during the period of 4:00 p.m. to 6:00 p.m.

Existing volumes and intersection geometry are shown on Figure 3. Traffic count data is included for reference in Appendix A.


HIGH PLAINS ESTATES

## Peak Hour Intersection Levels of Service - Existing Traffic

The Signalized and Unsignalized Intersection Analysis techniques, as published in the Highway Capacity Manual (HCM), 6th Edition, by the Transportation Research Board and as incorporated into the SYNCHRO computer program, were used to analyze the study intersections for existing and future traffic conditions. These nationally accepted techniques allow for the determination of intersection level of service (LOS) based on the congestion and delay of each traffic movement.

Level of service is a method of measurement used by transportation professionals to quantify a driver's perception of travel conditions that include travel time, number of stops, and total amount of stopped delay experienced on a roadway network. The HCM categorizes level of service into a range from "A" which indicates little, if any, vehicle delay, to "F" which indicates a level of operation considered unacceptable to most drivers. These levels of service grades with brief descriptions of the operating condition, for unsignalized and signalized intersections, are included for reference in Appendix B and have been used throughout this study.

The level of service analyses results for existing conditions are summarized in Table 1.
Intersection capacity worksheets developed for this study are provided in Appendix C.

Table 1 - Intersection Capacity Analysis Summary - Existing Traffic

| INTERSECTION | LEVEL OF SERVICE |  |
| :--- | :---: | :---: |
| LANE GROUPS | AM PEAK HOUR | PM PEAK HOUR |
| SH 60 / High Plains Boulevard (Stop-Controlled) |  |  |
| Eastbound Left | A | A |
| Southbound Left | C | C |
| Sounthbound Right | B | B |
| High Plains Boulevard / Brunner Boulevard (Stop-Controlled) |  |  |
| Westbound Left and Right | A | A |
| Southbound Left and Through | A | A |
| E County Road 14 / I-25 Frontage Road (Stop-Controlled) |  | A |
| Eastbound Left and Through | A | B |
| Southbound Left and Right | B |  |

Key: Stop-Controlled Intersection: Lev el of Service

## Existing Traffic Analysis Results

Under existing conditions, operational analysis shows that the unsignalized intersection of State Highway 60 with High Plains Boulevard has turning movement operations at or better than LOS C during both the morning and afternoon peak traffic hours.

The unsignalized intersection of High Plains Boulevard with Brunner Boulevard has turning movement operations at LOS A during both the morning and afternoon peak traffic hours.

The unsignalized intersection of E County Road 14 with I-25 Frontage Road has turning movement operations at LOS B or better during both the morning and afternoon peak traffic hours.

## III. Future Traffic Conditions Without Proposed Development

Background traffic is the traffic projected to be on area roadways without consideration of the proposed development. Background traffic includes traffic generated by development of vacant parcels in the area.

To account for projected increases in background traffic for Years 2024 and 2042, a compounded annual growth rate was determined using historical traffic data provided by CDOT's Online Transportation Information System (OTIS) along the adjacent segment of State Highway 60, which anticipates a 20-year growth rate of approximately two percent. Therefore, a growth rate of two percent was applied to existing traffic volumes. This annual growth rate provides for a conservative analysis and is assumed to account for regional growth projections and the level of in-fill development expected within the area.

To account for projected traffic from adjacent developments not yet built, trip generations from the Revere at Johnstown, Filing No. 1 traffic study ${ }^{1}$ were added to background traffic volumes. It is noted that additional development to the west and north of the proposed development site is anticipated pursuant to conceptual land use plans referred to as Great Plains Village. However, given the conceptual nature of this area, no specific traffic analyses are currently available. Therefore, future traffic volumes associated with this additional development is assumed to be accounted for within the applied two percent annual growth rate.

Pursuant to the area roadway improvements discussed in Section I, Year 2024 background traffic conditions assume the extension of High Plains Boulevard north to E County Road 14 as part of the adjacent Revere development including site access along this extension located opposite proposed site Access A (referred to as Access Drive). Year 2042 assumes no additional roadway improvements to accommodate regional transportation demands. This assumption provides for a conservative analysis.

## Background Traffic Signal Warrant

A signal warrant analysis, using Year 2024 and 2042 background traffic volumes, was conducted for the State Highway 60 intersection with High Plains Boulevard in order to review potential for traffic signal control. Year 2024 Analysis results conclude that the study intersection was found to be above the minimum vehicle volumes required to meet Warrant 3 - Peak Hour, from the Manual on Uniform Traffic Control Devices (MUTCD), for the installation of a traffic signal. It is noted however that warrants performed in the previous traffic study for adjacent development did not anticipate signalization by Year 2024. As such, the State Highway 60 and High Plains Boulevard intersection analysis remained a stop-controlled condition for Year 2024 but is assumed to be signalized by Year 2042. Warrant study worksheets are provided for reference in Appendix D.

[^28]Warrant 3 is intended for use at locations where traffic conditions are such that for a minimum of one hour on an average day, the minor-street (High Plains Boulevard) traffic suffers undue delay when entering or crossing the major street (State Highway 60). This assumption provides for a conservative analysis. Said study intersection should be monitored further by CDOT and County Staff as area development occurs to determine when signalization installation is appropriate.

Projected background traffic volumes and intersection geometry for Years 2024 and 2042 are shown on Figure 4 and Figure 5, respectively.


©


HIGH PLAINS ESTATES
Traffic Impact Study
SM ROCHA, LLC
Traffic and Transportation Consultants

## Peak Hour Intersection Levels of Service - Background Traffic

As with existing traffic conditions, the operations of study intersections were analyzed under background conditions, without the proposed development, using the SYNCHRO computer program.

Background traffic level of service analysis results for Year 2024 are listed in Table 2. Year 2042 operational results are summarized in Table 3.

Definitions of levels of service are given in Appendix B. Intersection capacity worksheets are provided in Appendix C.

Table 2 - Intersection Capacity Analysis Summary - Background Traffic - Year 2024

| INTERSECTION <br> LANE GROUPS | LEVEL OF SERVICE |  |
| :---: | :---: | :---: |
|  | AM PEAK HOUR | PM PEAK HOUR |
| SH 60 / High Plains Boulevard (Stop-Controlled) <br> Eastbound Left <br> Southbound Left <br> Sounthbound Right | $\begin{aligned} & \mathrm{A} \\ & \mathrm{C} \\ & \mathrm{C} \end{aligned}$ | $\begin{aligned} & \text { A } \\ & \text { E } \\ & \text { B } \end{aligned}$ |
| High Plains Boulevard / Brunner Boulevard (Stop-Controlled) <br> Westbound Left and Right <br> Southbound Left and Through | $\begin{aligned} & \text { A } \\ & \text { A } \end{aligned}$ | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~A} \end{aligned}$ |
| E County Road 14 / I-25 Frontage Road (Stop-Controlled) Eastbound Left and Through Southbound Left and Right | $\begin{aligned} & \text { A } \\ & \text { B } \end{aligned}$ | $\begin{aligned} & \text { A } \\ & \text { B } \end{aligned}$ |
| E County Road 14 / High Plains Boulevard (Stop-Controlled) <br> Westbound Left and Through <br> Northbound Left and Right | $\begin{aligned} & \text { A } \\ & \text { A } \end{aligned}$ | $\begin{aligned} & \text { A } \\ & \text { A } \end{aligned}$ |
| High Plains Boulevard / Access Drive (Stop-Controlled) <br> Eastbound Left <br> Eastbound Right <br> Northbound Left | $\begin{aligned} & \mathrm{A} \\ & \mathrm{~A} \\ & \mathrm{~A} \end{aligned}$ | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~A} \\ & \mathrm{~A} \\ & \hline \end{aligned}$ |

Key: Stop-Controlled Intersection: Lev el of Service

## Background Traffic Analysis Results - Year 2024

Year 2024 background traffic analysis indicates that the unsignalized intersection of State Highway 60 with High Plains Boulevard has turning movement operations at or better than LOS C during the AM peak traffic hour and LOS B or better during the PM peak traffic hour. Exceptions would include the southbound left turning movement which operates at LOS E during the PM peak traffic hour. The LOS E operation is attributed to the high through traffic volumes along State Highway 60 and the stopcontrolled nature of the intersection.

The unsignalized intersection of High Plains Boulevard with Brunner Boulevard has turning movement operations at LOS A during the AM peak traffic hour and LOS B or better during the PM peak traffic hour.

The unsignalized intersection of E County Road 14 with I-25 Frontage Road has turning movement operations at or better than LOS B during both the AM and PM peak traffic hours.

The unsignalized intersection of E County Road 14 with High Plains Boulevard has turning movement operations at LOS A during both the AM and PM peak traffic hours.

The unsignalized intersection of High Plains Boulevard with Access Drive has turning movement operations at LOS A during the AM peak traffic hour and LOS B or better during the PM peak traffic hour.

Table 3 - Intersection Capacity Analysis Summary - Background Traffic - Year 2042

| INTERSECTION | LEVEL OF SERVICE |  |
| :---: | :---: | :---: |
| LANE GROUPS | AM PEAK HOUR | PM PEAK HOUR |
| SH 60 / High Plains Boulevard (Signalized) | B (13.1) | A (8.1) |
| High Plains Boulevard / Brunner Boulevard (Stop-Controlled) <br> Westbound Left and Right <br> Southbound Left and Through | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~A} \end{aligned}$ | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~A} \end{aligned}$ |
| E County Road 14 / I-25 Frontage Road (Stop-Controlled) <br> Eastbound Left and Through <br> Southbound Left and Right | $\begin{aligned} & \text { A } \\ & \text { B } \end{aligned}$ | $\begin{aligned} & \text { A } \\ & \text { D } \end{aligned}$ |
| E County Road 14 / High Plains Boulevard (Stop-Controlled) <br> Westbound Left and Through <br> Northbound Left and Right | $\begin{aligned} & \text { A } \\ & \text { A } \end{aligned}$ | $\begin{aligned} & \text { A } \\ & \text { B } \end{aligned}$ |
| High Plains Boulevard / Access Drive (Stop-Controlled) <br> Eastbound Left <br> Eastbound Right <br> Northbound Left | $\begin{aligned} & \text { A } \\ & \text { A } \\ & \text { A } \end{aligned}$ | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~A} \\ & \mathrm{~A} \end{aligned}$ |

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)
Stop-Controlled Intersection: Lev el of Serv ice

## Background Traffic Analysis Results - Year 2042

By Year 2042 and without the proposed development, the study intersection of State Highway 60 with High Plains Boulevard experiences LOS B operations during the AM peak traffic hour and LOS A operations during the PM peak traffic hour.

The study intersection of High Plains Boulevard with Brunner Boulevard experiences LOS B or better operations during both the AM and PM peak traffic hours.

The study intersection of E County Road 14 with I-25 Frontage Road experiences LOS B or better operations during the AM peak traffic hour and LOS D or better operations during the PM peak traffic hour.

The study intersection of E County Road 14 with High Plains Boulevard experiences LOS A operations during the AM peak traffic hour and LOS B or better operations during the PM peak traffic hour.

The study intersection of High Plains Boulevard with Access Drive experiences LOS A operations during the AM peak traffic hour and LOS B or better operations during the PM peak traffic hour.

## IV. Proposed Project Traffic

## Trip Generation

Standard traffic generation characteristics compiled by the Institute of Transportation Engineers (ITE) in their report entitled Trip Generation Manual, $11^{\text {th }}$ Edition, were applied to the proposed land use in order to estimate average daily traffic (ADT), AM Peak Hour, and PM Peak Hour vehicle trips. A vehicle trip is defined as a one-way vehicle movement from a point of origin to a point of destination.

The ITE land use codes 210 (Single-Family Detached Housing) and 220 (Multifamily Housing (LowRise)) were used for estimating trip generation because of their best fit to the proposed land use descriptions.

Trip generation rates used in this study are presented in Table 4.

Table 4 - Trip Generation Rates

| $\begin{gathered} \text { ITE } \\ \text { CODE } \end{gathered}$ | LAND USE | UNIT | TRIP GENERATION RATES |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\begin{gathered} 24 \\ \text { HOUR } \end{gathered}$ | AM PEAK HOUR |  |  | PM PEAK HOUR |  |  |
|  |  |  |  | ENTER | EXIT | TOTAL | ENTER | EXIT | TOTAL |
| 210 | Single-Family Detached Housing | DU | 9.43 | 0.18 | 0.52 | 0.70 | 0.59 | 0.35 | 0.94 |
| 220 | Multifamily Housing (Low-Rise) | DU | 6.74 | 0.10 | 0.30 | 0.40 | 0.32 | 0.19 | 0.51 |

Key: $\quad \mathrm{DU}=$ Dwelling Units.
Table 5 illustrates projected ADT, AM Peak Hour, and PM Peak Hour traffic volumes likely generated by the proposed development upon build-out.

Table 5 - Trip Generation Summary

| $\begin{aligned} & \text { ITE } \\ & \text { CODE } \end{aligned}$ | LAND USE | SIZE | TOTAL TRIPS GENERATED |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\begin{gathered} 24 \\ \text { HOUR } \end{gathered}$ | AM PEAK HOUR |  |  | PM PEAK HOUR |  |  |
|  |  |  |  | ENTER | EXIT | TOTAL | ENTER | EXIT | TOTAL |
| 210 | Single-Family Detached Housing | 401 DU | 3,781 | 73 | 208 | 281 | 237 | 139 | 377 |
| 220 | Multifamily Housing (Low-Rise) | 226 DU | 1,523 | 22 | 69 | 90 | 73 | 43 | 115 |
|  |  | Total: | 5,305 | 95 | 276 | 371 | 310 | 182 | 492 |

Key: $\quad$ DU = Dw elling Units.
Note: All data and calculations above are subject to being rounded to nearest value.
Upon build-out, Table 5 illustrates that the proposed development has the potential to generate approximately 5,305 daily vehicle trips with 371 of those occurring during the morning peak hour and 492 during the afternoon peak hour.

## Adjustments to Trip Generation Rates

A development of this type is not likely to attract trips from within area land uses nor pass-by or diverted link trips from the adjacent roadway system, therefore no trip reduction was taken in this analysis.

## Trip Distribution

The overall directional distribution of site-generated traffic was determined based on the location of development site within the County, proposed and existing area land uses, allowed turning movements, available roadway network, and in reference to distribution patterns assumed in the previously prepared traffic study for the adjacent development.

Overall trip distribution patterns for the development are shown on Figure 6.

## Trip Assignment

Traffic assignment is how generated and distributed vehicle trips are expected to be loaded onto the available roadway network.

Applying trip distribution patterns to site-generated traffic provides the overall site-generated trip assignments shown on Figure 6.


## V. Future Traffic Conditions With Proposed Developments

Total traffic is the traffic projected to be on area roadways with consideration of the proposed development. Total traffic includes background traffic projections for Years 2024 and 2042 with consideration of site-generated traffic. For analysis purposes, it was assumed that development construction would be completed by end of Year 2024.

Pursuant to area roadway improvement discussions provided in Section III, Year 2024 and Year 2042 total traffic conditions assume no additional roadway improvements to accommodate regional transportation demands. Roadway improvements associated with site development are expected to be limited to site access and frontage as required by the governing agency.

As previously discussed in Section III, Year 2024 and Year 2042 background traffic conditions indicate that the State Highway 60 and High Plains Boulevard intersection was found to be above the minimum vehicle volumes required to meet Warrant 3 - Peak Hour, from the MUTCD, for the installation of a traffic signal. To be consistent with background traffic assumptions, it is expected that signalization will have occurred after Year 2024. As such, the intersection was analyzed under a stop-controlled condition for Year 2024 and with traffic signal control by Year 2042.

Projected Year 2024 total traffic volumes and intersection geometry are shown in Figure 7.
Figure 8 shows projected total traffic volumes and intersection geometry for Year 2042.



## VI. Project Impacts

The analyses and procedures described in this study were performed in accordance with the latest HCM and are based upon the worst-case conditions that occur during a typical weekday upon buildout of site development and analyzed land uses. Therefore, study intersections are likely to operate with traffic conditions better than those described within this study, which represent the peak hours of weekday operations only.

## Peak Hour Intersection Levels of Service - Total Traffic

As with background traffic, the operations of the study intersections were analyzed under projected total traffic conditions using the SYNCHRO computer program. Total traffic level of service analysis results for Years 2024 and 2042 are summarized in Table 6 and Table 7, respectively.

Definitions of levels of service are given in Appendix B. Intersection capacity worksheets are provided in Appendix C.

Table 6 - Intersection Capacity Analysis Summary - Total Traffic - Year 2024

| INTERSECTION | LEVEL OF SERVICE |  |
| :---: | :---: | :---: |
| LANE GROUPS | AM PEAK HOUR | PM PEAK HOUR |
| SH 60 / High Plains Boulevard (Stop-Controlled) <br> Eastbound Left <br> Southbound Left <br> Sounthbound Right | $\begin{aligned} & A \\ & D \\ & D \end{aligned}$ | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~F} \\ & \mathrm{~B} \end{aligned}$ |
| High Plains Boulevard / Brunner Boulevard (Stop-Controlled) <br> Westbound Lef and Right <br> Southbound Left and Through | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~A} \end{aligned}$ | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~A} \end{aligned}$ |
| E County Road 14 / I-25 Frontage Road (Stop-Controlled) <br> Eastbound Left and Through <br> Southbound Left and Right | $\begin{aligned} & \text { A } \\ & \text { B } \end{aligned}$ | $\begin{aligned} & \text { A } \\ & \text { C } \end{aligned}$ |
| E County Road 14 / High Plains Boulevard (Stop-Controlled) <br> Westbound Left and Through <br> Northbound Left and Right | $\begin{aligned} & \text { A } \\ & \text { B } \end{aligned}$ | $\begin{aligned} & \text { A } \\ & \text { B } \end{aligned}$ |
| High Plains Boulevard / Access A (Stop-Controlled) <br> Eastbound Left and Through <br> Eastbound Right <br> Westbound Left and Through <br> Westbound Right <br> Northbound Left <br> Southbound Left, Through and Right | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~A} \\ & \mathrm{~B} \\ & \mathrm{~A} \\ & \mathrm{~A} \\ & \mathrm{~A} \\ & \hline \end{aligned}$ | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~A} \\ & \mathrm{C} \\ & \mathrm{~A} \\ & \mathrm{~A} \\ & \mathrm{~A} \end{aligned}$ |
| E County Road 14 / Access B (Stop-Controlled) <br> Westbound Left and Through <br> Northbound Left and Right | $\begin{aligned} & \text { A } \\ & \text { B } \end{aligned}$ | $\begin{aligned} & \mathrm{A} \\ & \mathrm{~B} \\ & \hline \end{aligned}$ |

[^29]Table 7 - Intersection Capacity Analysis Summary - Total Traffic - Year 2042

| INTERSECTION LANE GROUPS | LEVEL OF SERVICE |  |
| :---: | :---: | :---: |
|  | AM PEAK HOUR | PM PEAK HOUR |
| SH 60 / High Plains Boulevard (Signalized) | C (23.3) | B (10.1) |
| High Plains Boulevard / Brunner Boulevard (Stop-Controlled) <br> Westbound Left and Right <br> Southbound Left and Through | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~A} \end{aligned}$ | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~A} \end{aligned}$ |
| E County Road 14 / I-25 Frontage Road (Stop-Controlled) <br> Eastbound Left and Through <br> Southbound Left and Right | $\begin{aligned} & A \\ & C \end{aligned}$ | $\begin{aligned} & \text { A } \\ & \text { E } \end{aligned}$ |
| E County Road 14 / High Plains Boulevard (Stop-Controlled) <br> Westbound Left and Through <br> Northbound Left and Right | $\begin{aligned} & \text { A } \\ & \text { B } \end{aligned}$ | $\begin{aligned} & \text { A } \\ & \text { B } \end{aligned}$ |
| High Plains Boulevard / Access A (Stop-Controlled) <br> Eastbound Left and Through <br> Eastbound Right <br> Westbound Left and Through <br> Westbound Right <br> Northbound Left <br> Southbound Left, Through and Right | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~A} \\ & \mathrm{~B} \\ & \mathrm{~A} \\ & \mathrm{~A} \\ & \mathrm{~A} \end{aligned}$ | $\begin{aligned} & \mathrm{B} \\ & \mathrm{~A} \\ & \mathrm{C} \\ & \mathrm{~A} \\ & \mathrm{~A} \\ & \mathrm{~A} \end{aligned}$ |
| E County Road 14 / Access B (Stop-Controlled) <br> Westbound Left and Through Northbound Left and Right | $\begin{aligned} & \text { A } \\ & \text { B } \end{aligned}$ | $\begin{aligned} & A \\ & C \end{aligned}$ |

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)
Stop-Controlled Intersection: Lev el of Serv ice

## Total Traffic Analysis Results Upon Development Build-Out

Table 7 illustrates how, by Year 2042 and upon development build-out, the signalized intersection of State Highway 60 with High Plains Boulevard shows an overall LOS C operation during the morning peak traffic hour and LOS B operation during the afternoon peak traffic hour. Compared to the background traffic analysis, the traffic generated by the proposed development is not expected to significantly change the operations of the study intersection.

The stop-controlled intersection of High Plains Boulevard with Brunner Boulevard is projected to have turning movement operations at LOS B or better for both the morning and afternoon peak traffic hours.

The stop-controlled intersection of E County Road 14 with I-25 Frontage Road is projected to have turning movement operations at LOS C or better for the morning peak traffic hour and LOS A for the afternoon peak traffic hour. Exceptions would include the southbound turning movements and which operate at LOS E during the PM peak traffic hour. The LOS E operations are attributed to the high through traffic volumes along E County Road 14 and the stop-controlled nature of the intersection.

It is to be noted that it is not uncommon for unsignalized movements to or from an arterial roadway, in urban areas, to operate with noticeable delays during peak traffic hours. It is recommended that CDOT and County Staff continue to monitor the E County Road 14 and I-25 Frontage Road intersection as area development occurs to determine when mitigation measures may be most appropriate.

The stop-controlled intersection of E County Road 14 with High Plains Boulevard is projected to have turning movement operations at LOS B or better for both the morning and afternoon peak traffic hours.

The stop-controlled intersection of High Plains Boulevard with Access A is projected to have turning movement operations at LOS B or better for the morning peak traffic hour and LOS C or better for the afternoon peak traffic hour.

The stop-controlled intersection of E County Road 14 with Access B is projected to have turning movement operations at LOS B or better for the morning peak traffic hour and LOS C or better for the afternoon peak traffic hour.

These intersection operations are similar to background conditions.
As discussed in Section I, it is noted that proposed internal site access to the existing adjacent subdivision via River Rock Drive is anticipated to provide operations comparable to or better than the adjacent study intersections. Due to the acceptable operations shown at the intersection of High Plains Boulevard with Brunner Boulevard, no significant impacts to intersections along Brunner Boulevard or River Rock Drive are anticipated. Furthermore, projected average daily traffic volumes at the River Rock Drive access, as shown on Figures 7 and 8, are estimated to be approximately ten percent of total daily traffic volumes generated by the development and are considered to be minor.

## Auxiliary Lane Analysis

Auxiliary lanes for site development accesses are to be based on County's Engineering and Construction Criteria (Criteria) ${ }^{2}$, and CDOT's State Highway Access Code (SHAC) ${ }^{3}$.

Considering development build-out, an evaluation of auxiliary lane requirements, pursuant to Section 8.7, Table $8-4$ of the County's Criteria, as well as section 3.10 of the CDOT SHAC, reveals that left turn and right-turn deceleration lanes at Access A along High Plains Boulevard and Access B along E County Road 14 are required since the development's projected peak hour left turn and right turn ingress volumes exceed the County's thresholds of 10 and 25 vehicles per hour, respectively.

[^30]
## VII. Conclusion

This traffic impact study is provided as a planning document and addresses the capacity, geometric, and control requirements associated with the development entitled High Plains Estates. This proposed development consists of a residential subdivision including a mix of single-family and multifamily housing. The development is located on the south side of E County Road 14 and west of High Plains Boulevard in Weld County, Colorado.

The study area examined in this analysis encompassed High Plains Boulevard between E County Road 14 and State Highway 60, and proposed site access drives.

Analysis was conducted for critical AM Peak Hour and PM Peak Hour traffic operations for existing traffic conditions, Year 2024 and Year 2042 background traffic conditions, and Year 2024 and Year 2042 total traffic conditions.

Under existing conditions, operational analysis shows that the unsignalized study intersections generally experience turning movement operations at or better than LOS C during both the morning and afternoon peak traffic hours.

Year 2024 background traffic analysis indicates that the unsignalized intersection of State Highway 60 with High Plains Boulevard has turning movement operations at or better than LOS C during the AM peak traffic hour and LOS B or better during the PM peak traffic hour. Exceptions would include the southbound left turning movement which operates at LOS E during the PM peak traffic hour. The LOS E operation is attributed to the high through traffic volumes along State Highway 60 and the stopcontrolled nature of the intersection. The remaining unsignalized intersections have turning movement operations at or better than LOS B during either peak traffic hour.

By Year 2042 and without the proposed development, the signalized intersection of State Highway 60 with High Plains Boulevard experiences LOS B operations during the AM peak traffic hour and LOS A operations during the PM peak traffic hour. Stop-controlled study intersections experience LOS B or better operations during the AM peak traffic hour and LOS D or better during the PM peak traffic hour.

Analysis of future traffic conditions indicates that the addition of site-generated traffic is expected to create no negative impact to traffic operations for the existing and surrounding roadway system upon consideration of the various roadway and intersection control improvements assumed within this analysis. With all conservative assumptions defined in this analysis, the study intersections are projected to operate at future levels of service comparable to Year 2042 background traffic conditions. Proposed site accesses have long-term operations at LOS C or better during peak traffic periods and upon build-out.

## APPENDIX A

Traffic Count Data

Location: 1 HIGH PLAINS BLVD \& BRUNNER BLVD AM
Date: Wednesday, June 1, 2022
Peak Hour: 08:00 AM - 09:00 AM
(303) 216-2439 www.alltrafficdata.net

Peak 15-Minutes: 08:30 AM - 08:45 AM

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval | BRUNNER BLVD <br> Eastbound |  |  |  | BRUNNER BLVD <br> Westbound |  |  |  | HIGH PLAINS BLVD <br> Northbound |  |  |  |  | HIGH PLAINS BLVD <br> Southbound |  |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | Left | Thru R | Right |  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  |  | West | East | South |  |
| 7:00 AM | 0 | 0 | 0 | 0 | 0 | 17 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 18 | 74 | 0 | 2 | 0 | 0 |
| 7:15 AM | 0 | 0 | 0 | 0 | 0 | 15 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 17 | 76 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 0 | 0 | 0 | 0 | 19 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 22 | 78 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 0 | 0 | 0 | 0 | 15 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 17 | 80 | 0 | 2 | 0 | 0 |
| 8:00 AM | 0 | 0 | 0 | 0 | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 20 | 87 | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 9 | 0 | 0 | 0 | 0 | 0 | 19 |  | 0 | 2 | 0 | 0 |
| 8:30 AM | 0 | 0 | 0 | 0 | 0 | 20 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 24 |  | 0 | 2 | 0 | 0 |
| 8:45 AM | 0 | 0 | 0 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 24 |  | 0 | 1 | 0 | 0 |
| Count Total | 0 | 0 | 0 | 0 | 0 | 123 | 0 | 0 | 0 | 0 | 0 | 0 | 38 | 0 | 0 | 0 | 0 | 0 | 161 |  | 0 | 9 | 0 | 0 |
| Peak Hour | 0 | 0 | 0 | 0 | 0 | 57 | 0 | 0 | 0 | 0 | 0 | 0 | 30 | 0 | 0 |  | 0 | 0 |  | 87 | 0 | 5 | 0 | 0 |

Location: 1 HIGH PLAINS BLVD \& BRUNNER BLVD PM
Date: Wednesday, June 1, 2022
Peak Hour: 04:00 PM - 05:00 PM
(303) 216-2439 www.alltrafficdata.net

Peak 15-Minutes: 04:45 PM - 05:00 PM

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.

## Traffic Counts



Location: 2 HIGH PLAINS BLVD \& SH 60 AM
(303) 216-2439 www.alltrafficdata.net

Date: Wednesday, June 1, 2022
Peak Hour: 07:00 AM - 08:00 AM
Peak 15-Minutes: 07:15 AM - 07:30 AM


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval | $\text { SH } 60$ <br> Eastbound |  |  |  | $\text { SH } 60$ <br> Westbound |  |  |  | HIGH PLAINS BLVD <br> Northbound |  |  |  |  | HIGH PLAINS BLVD <br> Southbound |  |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | eft | Thru | Right | U-Turn | Left | Thru | Right |  | U-Turn | Left | Thru |  | Right |  |  | West | East | South |  |
| 7:00 AM | 0 | 1 | 48 | 0 | 0 | 0 | 137 | 1 | 0 | 0 | 0 |  | 0 | 0 | 3 | 0 | 0 | 24 | 214 | 910 | 0 | 0 | 0 | 0 |
| 7:15 AM | 0 | 6 | 56 | 0 | 0 | 0 | 151 | 3 | 0 | 0 | 0 |  | 0 | 0 | 6 | 0 | 0 | 21 | 243 | 884 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 5 | 51 | 0 | 0 | 0 | 131 | 4 | 0 | 0 | 0 |  | 0 | 0 | 4 | 0 | 0 | 29 | 224 | 843 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 1 | 65 | 0 | 0 | 0 | 137 | 2 | 0 | 0 | 0 |  | 0 | 0 | 6 | 0 | 0 | 18 | 229 | 823 | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 8 | 51 | 0 | 0 | 0 | 103 | 4 | 0 | 0 | 0 |  | 0 | 0 | 4 | 0 | 0 | 18 | 188 | 764 | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 8 | 58 | 0 | 0 | 0 | 110 | 5 | 0 | 0 | 0 |  | 0 | 0 | 3 | 0 | 0 | 18 | 202 |  | 0 | 0 | 0 | 0 |
| 8:30 AM | 0 | 2 | 55 | 0 | 0 | 0 | 110 | 8 | 0 | 0 | 0 |  | 0 | 0 | 9 | 0 | 0 | 20 | 204 |  | 0 | 0 | 0 | 0 |
| 8:45 AM | 0 | 9 | 57 | 0 | 0 | 0 | 82 | 3 | 0 | 0 | 0 |  | 0 | 0 | 4 | 0 | 0 | 15 | 170 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 40 | 441 | 0 | 0 | 0 | 961 | 30 | 0 | 0 | 0 |  | 0 | 0 | 39 | 0 | 0 | 163 | 1,674 |  | 0 | 0 | 0 | 0 |
| Peak Hour | 0 | 13 | 220 | 0 | 0 | 0 | 556 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 19 |  | 0 | 92 | 91 | 10 | 0 | 0 | 0 | 0 |

Date: Wednesday, June 1, 2022
Peak Hour: 04:00 PM - 05:00 PM
Peak 15-Minutes: 04:15 PM - 04:30 PM

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts


Location: 5 I 25 E FRONTAGE RD \& CR 14 AM
Date: Wednesday, June 1, 2022
Peak Hour: 07:00 AM - 08:00 AM
(303) 216-2439 www.alltrafficdata.net

Peak 15-Minutes: 07:00 AM - 07:15 AM

## Peak Hour - All Vehicles



Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.

## Traffic Counts



Location: 5 I 25 E FRONTAGE RD \& CR 14 PM
Date: Wednesday, June 1, 2022
Peak Hour: 04:00 PM - 05:00 PM
(303) 216-2439 www.alltrafficdata.net

Peak 15-Minutes: 04:30 PM - 04:45 PM

## Peak Hour - All Vehicles



## Peak Hour - Pedestrians/Bicycles on Crosswalk



Note: Total study counts contained in parentheses.

## Traffic Counts

| Interval | CR 14 <br> Eastbound |  |  |  | CR 14 <br> Westbound |  |  |  | 125 E FRONTAGE RD <br> Northbound |  |  |  | I 25 E FRONTAGE RD Southbound |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | eft | Thru R | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 4:00 PM | 0 | 13 | 50 | 1 | 0 | 0 | 37 | 10 | 0 | 0 | 0 | 2 | 0 | 8 | 0 | 22 | 143 | 603 | 1 | 0 | 1 | 0 |
| 4:15 PM | 0 | 14 | 40 | 0 | 0 | 1 | 35 | 10 | 0 | 0 | 0 | 0 | 1 | 12 | 0 | 21 | 134 |  | 0 | 0 | 0 | 0 |
| 4:30 PM | 0 | 16 | 49 | 0 | 0 | 0 | 33 | 9 | 0 | 0 | 0 | 0 | 0 | 30 | 0 | 37 | 174 |  | 0 | 0 | 0 | 0 |
| 4:45 PM | 0 | 11 | 44 | 0 | 0 | 3 | 30 | 8 | 0 | 0 | 0 | 0 | 0 | 26 | 0 | 30 | 152 |  | 0 | 0 | 1 | 0 |
| Count Total | 0 | 54 | 183 | 1 | 0 | 4 | 135 | 37 | 0 | 0 | 0 | 2 | 1 | 76 | 0 | 110 | 603 |  | 1 | 0 | 2 | 0 |
| Peak Hour | 0 | 54 | 183 | 1 | 0 | 4 | 135 | 37 | 0 | 0 | 0 | 2 | 1 | 76 |  | $0 \quad 110$ | 603 | 3 | 1 | 0 | 2 | 0 |


All Traffic Data Services, LLC

## APPENDIX B

Level of Service Definitions

The following information can be found in the Highway Capacity Manual, Transportation Research Board, 2016: Chapter 19 - Signalized Intersections and Chapter 20 - Two-Way Stop Controlled Intersections.

## Automobile Level of Service (LOS) for Signalized Intersections

Levels of service are defined to represent reasonable ranges in control delay.

## LOS A

Describes operations with a control delay of $10 \mathrm{~s} / \mathrm{veh}$ or less and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is exceptionally favorable or the cycle length is very short. If it is due to favorable progression, most vehicles arrive during the green indication and travel through the intersection without stopping.

## LOS B

Describes operations with control delay between 10 and 20 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is highly favorable or the cycle length is short. More vehicles stop than with LOS A.

## LOS C

Describes operations with control delay between 20 and $35 \mathrm{~s} / \mathrm{veh}$ and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when progression is favorable or the cycle length is moderate. Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear at this level. The number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.

## LOS D

Describes operations with control delay between 35 and $55 \mathrm{~s} / \mathrm{veh}$ and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high and either progression is ineffective or the cycle length is long. Many vehicles stop and individual cycle failures are noticeable.

## LOS E

Describes operations with control delay between 55 and 80 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high, progression is unfavorable, and the cycle length is long. Individual cycle failures are frequent.

LOS F
Describes operations with control delay exceeding $80 \mathrm{~s} / \mathrm{veh}$ or a volume-to-capacity ratio greater than 1.0 . This level is typically assigned when the volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.

## Level of Service (LOS) for Unsignalized TWSC Intersections

| Level of Service (v/c $\leq 1.0)$ | Average Control Delay (s/veh) |
| :---: | :---: |
| A | $0-10$ |
| B | $>10-15$ |
| C | $>15-25$ |
| D | $>25-35$ |
| E | $>35-50$ |
| F | $>50$ |

## APPENDIX C

## Capacity Worksheets

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.9 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | $\mathbf{1}$ | $\mathbf{4}$ | $\mathbf{4}$ | $\mathbf{7}$ | $\mathbf{1}$ | $\mathbf{7}$ |
| Traffic Vol, veh/h | 13 | 220 | 556 | 10 | 19 | 92 |
| Future Vol, veh/h | 13 | 220 | 556 | 10 | 19 | 92 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 520 | - | - | 375 | 0 | 0 |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 14 | 239 | 604 | 11 | 21 | 100 |


| Major/Minor | Major1 | Major2 |  |  | Minor2 |  |  |
| :--- | ---: | :--- | :--- | :--- | ---: | ---: | :---: |
| Conflicting Flow All | 615 | 0 | - | 0 | 871 | 604 |  |
| Stage 1 | - | - | - | - | 604 | - |  |
| Stage 2 | - | - | - | - | 267 | - |  |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |  |
| Follow-up Hdwy | 2.218 | - | - | -3.518 | 3.318 |  |  |
| Pot Cap-1 Maneuver | 965 | - | - | - | 322 | 498 |  |
| $\quad$ Stage 1 | - | - | - | - | 546 | - |  |
| Stage 2 | - | - | - | - | 778 | - |  |
| Platoon blocked, \% |  | - | - | - |  |  |  |
| Mov Cap-1 Maneuver | 965 | - | - | - | 317 | 498 |  |
| Mov Cap-2 Maneuver | - | - | - | - | 317 | - |  |
| Stage 1 | - | - | - | - | 538 | - |  |
| Stage 2 | - | - | - | - | 778 | - |  |


| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 0.5 | 0 | 14.5 |
| HCM LOS |  |  | B |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 965 | - | - | - | 317 | 498 |
| HCM Lane V/C Ratio | 0.015 | - | - | -0.065 | 0.201 |  |
| HCM Control Delay (s) | 8.8 | - | - | - | 17.1 | 14 |
| HCM Lane LOS | A | - | - | - | $C$ | B |
| HCM 95th \%tile Q(veh) | 0 | - | - | - | 0.2 | 0.7 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 5.7 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | M |  | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 57 | 0 | 0 | 30 | 0 | 0 |
| Future Vol, veh/h | 57 | 0 | 0 | 30 | 0 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 62 | 0 | 0 | 33 | 0 | 0 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.3 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\mathbf{A}$ | $\mathbf{F}$ |  | Mr |  |
| Traffic Vol, veh/h | 32 | 77 | 137 | 25 | 49 | 40 |
| Future Vol, veh/h | 32 | 77 | 137 | 25 | 49 | 40 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 35 | 84 | 149 | 27 | 53 | 43 |


| Major/Minor | Major1 | Major2 |  | Minor2 |  |  |
| :---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Conflicting Flow All | 176 | 0 | - | 0 | 317 | 163 |
| Stage 1 | - | - | - | - | 163 | - |
| Stage 2 | - | - | - | - | 154 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | -3.518 | 3.318 |  |
| Pot Cap-1 Maneuver | 1400 | - | - | - | 676 | 882 |
| $\quad$ Stage 1 | - | - | - | - | 866 | - |
| Stage 2 | - | - | - | - | 874 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1400 | - | - | - | 658 | 882 |
| Mov Cap-2 Maneuver | - | - | - | - | 658 | - |
| Stage 1 | - | - | - | - | 843 | - |
| Stage 2 | - | - | - | - | 874 | - |


| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 2.2 | 0 | 10.6 |
| HCM LOS |  | $B$ |  |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1400 | - | - | - | 743 |
| HCM Lane V/C Ratio | 0.025 | - | - | - | 0.13 |
| HCM Control Delay (s) | 7.6 | 0 | - | -10.6 |  |
| HCM Lane LOS | A | A | - | - | B |
| HCM 95th \%tile Q(veh) | 0.1 | - | - | - | 0.4 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.9 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | $\mathbf{1}$ | 4 | $\mathbf{4}$ | $\mathbf{7}$ | $\mathbf{1}$ | $\mathbf{7}$ |
| Traffic Vol, veh/h | 93 | 510 | 301 | 36 | 29 | 37 |
| Future Vol, veh/h | 93 | 510 | 301 | 36 | 29 | 37 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 520 | - | - | 375 | 0 | 0 |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 101 | 554 | 327 | 39 | 32 | 40 |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 366 | 0 | - | 0 | 1083 | 327 |  |
| Stage 1 | - | - | - | - | 327 | - |  |
| Stage 2 | - | - | - | - | 756 | - |  |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |  |
| Critical Hdwy Stg 1 |  | - | - | - | 5.42 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |  |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |  |
| Pot Cap-1 Maneuver | 1193 | - | - | - | 240 | 714 |  |
| Stage 1 | - | - | - | - | 731 | - |  |
| Stage 2 | - | - | - | - | 464 | - |  |
| Platoon blocked, \% |  | - | - | - |  |  |  |
| Mov Cap-1 Maneuver | 1193 | - | - | - | 220 | 714 |  |
| Mov Cap-2 Maneuver | - | - | - | - | 220 | - |  |
| Stage 1 | - | - | - | - | 669 | - |  |
| Stage 2 | - | - | - | - | 464 | - |  |
|  |  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |  |
| HCM Control Delay, s | 1.3 |  | 0 |  | 16.4 |  |  |
| HCM LOS |  |  |  |  | C |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT WBR SBLn1 SBLn2 |  |  |  |
| Capacity (veh/h) |  | 1193 | - | - | - | 220 | 714 |
| HCM Lane V/C Ratio |  | 0.085 | - | - | - | 0.143 | 0.056 |
| HCM Control Delay (s) |  | 8.3 | - | - | - | 24.1 | 10.3 |
| HCM Lane LOS |  | A | - | - | - | C | B |
| HCM 95th \%tile Q(veh) |  | 0.3 | - | - | - | 0.5 | 0.2 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.8 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 45 | 1 | 2 | 61 | 1 | 1 |
| Future Vol, veh/h | 45 | 1 | 2 | 61 | 1 | 1 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 49 | 1 | 2 | 66 | 1 | 1 |


| Major/Minor M | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 38 | 35 | 0 | 0 | 68 | 0 |
| Stage 1 | 35 | - | - | - | - | - |
| Stage 2 | 3 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 |  | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 974 | 1038 | - | - | 1533 | - |
| Stage 1 | 987 | - | - | - | - | - |
| Stage 2 | 1020 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 973 | 1038 | - | - | 1533 | - |
| Mov Cap-2 Maneuver | 973 | - | - | - | - | - |
| Stage 1 | 987 | - | - | - | - | - |
| Stage 2 | 1019 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 8.9 |  | 0 |  | 3.7 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL |  |
| Capacity (veh/h) |  | - | - | 974 | 1533 | - |
| HCM Lane V/C Ratio |  | - | - | 0.051 | 0.001 | - |
| HCM Control Delay (s) |  | - | - | 8.9 | 7.3 | 0 |
| HCM Lane LOS |  | - | - | A | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 0.2 | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 4.6 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\mathbf{4}$ | $\mathbf{b}$ |  | Mr |  |
| Traffic Vol, veh/h | 54 | 183 | 135 | 37 | 77 | 110 |
| Future Vol, veh/h | 54 | 183 | 135 | 37 | 77 | 110 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 59 | 199 | 147 | 40 | 84 | 120 |


| Major/Minor | Major1 | Major2 |  |  |  |  |
| :--- | ---: | :--- | :--- | :--- | :--- | ---: |


| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 1.8 | 0 | 12.5 |
| HCM LOS |  |  | B |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1387 | - | - | -681 |
| HCM Lane V/C Ratio | 0.042 | - | - | -0.298 |
| HCM Control Delay (s) | 7.7 | 0 | - | -12.5 |
| HCM Lane LOS | A | A | - | - |
| HCM $95 t h$ \%tile Q(veh) | 0.1 | - | - | - |


| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 4.1 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL |  |
| Lane Configurations | ${ }^{7}$ | 4 | 4 | F | ${ }^{7}$ | 「 |
| Traffic Vol, veh/h | 48 | 229 | 578 | 16 | 37 | 184 |
| Future Vol, veh/h | 48 | 229 | 578 | 16 | 37 | 184 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 520 | - | - | 375 | 0 | 0 |
| Veh in Median Storage, \# | \# - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mumt Flow | 52 | 249 | 628 | 17 | 40 | 200 |


| Major/Minor | Major1 | Major2 |  |  | Minor2 |  |  |
| :---: | ---: | :---: | :---: | :---: | ---: | ---: | :---: |
| Conflicting Flow All | 645 | 0 | - | 0 | 981 | 628 |  |
| Stage 1 | - | - | - | - | 628 | - |  |
| Stage 2 | - | - | - | - | 353 | - |  |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |  |
| Follow-up Hdwy | 2.218 | - | - | -3.518 | 3.318 |  |  |
| Pot Cap-1 Maneuver | 940 | - | - | - | 277 | 483 |  |
| $\quad$ Stage 1 | - | - | - | - | 532 | - |  |
| Stage 2 | - | - | - | - | 711 | - |  |
| Platoon blocked, \% |  | - | - | - |  |  |  |
| Mov Cap-1 Maneuver | 940 | - | - | - | 262 | 483 |  |
| Mov Cap-2 Maneuver | - | - | - | - | 262 | - |  |
| Stage 1 | - | - | - | - | 503 | - |  |
| Stage 2 | - | - | - | - | 711 | - |  |


| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 1.6 | 0 | 18.2 |
| HCM LOS |  |  | C |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 940 | - | - | - | 262 | 483 |
| HCM Lane V/C Ratio | 0.056 | - | - | - | 0.154 | 0.414 |
| HCM Control Delay (s) | 9.1 | - | - | - | 21.2 | 17.6 |
| HCM Lane LOS | A | - | - | - | C | C |
| HCM 95th \%tile Q(veh) | 0.2 | - | - | - | 0.5 | 2 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.5 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{F}$ |  | $\mathbf{F}$ |  |  | $\boldsymbol{\uparrow}$ |
| Traffic Vol, veh/h | 59 | 0 | 40 | 31 | 0 | 105 |
| Future Vol, veh/h | 59 | 0 | 40 | 31 | 0 | 105 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 64 | 0 | 43 | 34 | 0 | 114 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 174 | 60 | 0 | 0 | 77 | 0 |
| Stage 1 | 60 | - | - | - | - | - |
| Stage 2 | 114 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 |  | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 816 | 1005 | - | - | 1522 | - |
| Stage 1 | 963 | - | - | - | - | - |
| Stage 2 | 911 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 816 | 1005 | - | - | 1522 | - |
| Mov Cap-2 Maneuver | 816 | - | - | - | - | - |
| Stage 1 | 963 | - | - | - | - | - |
| Stage 2 | 911 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 9.8 |  | 0 |  | 0 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 816 | 1522 | - |
| HCM Lane V/C Ratio |  | - | - | 0.079 | - | - |
| HCM Control Delay (s) |  | - | - | 9.8 | 0 | - |
| HCM Lane LOS |  | - | - | A | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0.3 | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.1 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\mathbf{4}$ | $\mathbf{i}$ |  | MF |  |
| Traffic Vol, veh/h | 33 | 91 | 177 | 35 | 54 | 42 |
| Future Vol, veh/h | 33 | 91 | 177 | 35 | 54 | 42 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 36 | 99 | 192 | 38 | 59 | 46 |


| Major/Minor | Major1 | Major2 |  |  | Minor2 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | :---: |
| Conflicting Flow All | 230 | 0 | - | 0 | 382 | 211 |  |
| Stage 1 | - | - | - | - | 211 | - |  |
| Stage 2 | - | - | - | - | 171 | - |  |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |  |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |  |
| Pot Cap-1 Maneuver | 1338 | - | - | - | 620 | 829 |  |
| $\quad$ Stage 1 | - | - | - | - | 824 | - |  |
| Stage 2 | - | - | - | - | 859 | - |  |
| Platoon blocked, \% |  | - | - | - |  |  |  |
| Mov Cap-1 Maneuver | 1338 | - | - | - | 603 | 829 |  |
| Mov Cap-2 Maneuver | - | - | - | - | 603 | - |  |
| Stage 1 | - | - | - | - | 801 | - |  |
| Stage 2 | - | - | - | - | 859 | - |  |


| Approach | EB | WB | SB |
| :--- | :---: | :---: | :---: |
| HCM Control Delay, s | 2.1 | 0 | 11.2 |
| HCM LOS |  | $B$ |  |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1338 | - | - | -685 |
| HCM Lane V/C Ratio | 0.027 | - | - | -0.152 |
| HCM Control Delay (s) | 7.8 | 0 | - | -11.2 |
| HCM Lane LOS | A | A | - | - |
| HCM $95 t h$ \%tile Q(veh) | 0.1 | - | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.3 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\mathbf{7}$ |  |  | $\mathbf{\uparrow}$ | Mr |  |
| Traffic Vol, veh/h | 139 | 0 | 4 | 221 | 0 | 11 |
| Future Vol, veh/h | 139 | 0 | 4 | 221 | 0 | 11 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, $\#$ | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 151 | 0 | 4 | 240 | 0 | 12 |


| Major/Minor | Major1 | Major2 |  |  |  |  |  | Minor1 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 151 | 0 | 399 | 151 |  |  |  |  |
| Stage 1 | - | - | - | - | 151 | - |  |  |  |  |
| Stage 2 | - | - | - | - | 248 | - |  |  |  |  |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |  |  |  |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |  |  |  |  |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |  |  |  |  |
| Follow-up Hdwy | - | - | 2.218 | - | 3.518 | 3.318 |  |  |  |  |
| Pot Cap-1 Maneuver | - | - | 1430 | - | 607 | 895 |  |  |  |  |
| $\quad$ Stage 1 | - | - | - | - | 877 | - |  |  |  |  |
| Stage 2 | - | - | - | - | 793 | - |  |  |  |  |
| Platoon blocked, \% | - | - |  | - |  |  |  |  |  |  |
| Mov Cap-1 Maneuver | - | - | 1430 | - | 605 | 895 |  |  |  |  |
| Mov Cap-2 Maneuver | - | - | - | - | 605 | - |  |  |  |  |
| Stage 1 | - | - | - | - | 877 | - |  |  |  |  |
| Stage 2 | - | - | - | - | 791 | - |  |  |  |  |


| Approach | EB | WB | NB |
| :--- | ---: | :---: | ---: |
| HCM Control Delay, s | 0 | 0.1 | 9.1 |
| HCM LOS |  |  | A |


| Minor Lane/Major Mvmt | NBLn1 | EBT | EBR | WBL | WBT |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 895 | - | -1430 | - |  |
| HCM Lane V/C Ratio | 0.013 | - | -0.003 | - |  |
| HCM Control Delay (s) | 9.1 | - | - | 7.5 | 0 |
| HCM Lane LOS | A | - | - | A | A |
| HCM 95th \%tile Q(veh) | 0 | - | - | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 8.1 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | T | $\mathbf{7}$ |  | 4 | $\mathbf{F}$ |  |
| Traffic Vol, veh/h | 11 | 105 | 40 | 0 | 0 | 4 |
| Future Vol, veh/h | 11 | 105 | 40 | 0 | 0 | 4 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | 0 | 150 | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 12 | 114 | 43 | 0 | 0 | 4 |





| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\uparrow$ |  |  | $\neq 1$ |
| Traffic Vol, veh/h | 47 | 1 | 129 | 63 | 1 | 69 |
| Future Vol, veh/h | 47 | 1 | 129 | 63 | 1 | 69 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 51 | 1 | 140 | 68 | 1 | 75 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 251 | 174 | 0 | 0 | 208 | 0 |
| Stage 1 | 174 | - | - | - | - | - |
| Stage 2 | 77 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 |  | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 738 | 869 | - | - | 1363 | - |
| Stage 1 | 856 | - | - | - | - | - |
| Stage 2 | 946 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 737 | 869 | - | - | 1363 | - |
| Mov Cap-2 Maneuver | 737 | - | - | - | - | - |
| Stage 1 | 856 | - | - | - | - | - |
| Stage 2 | 945 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 10.2 |  | 0 |  | 0.1 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 739 | 1363 | - |
| HCM Lane V/C Ratio |  | - | - | 0.071 | 0.001 | - |
| HCM Control Delay (s) |  | - | - | 10.2 | 7.6 | 0 |
| HCM Lane LOS |  | - | - | B | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 0.2 | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 4.8 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\mathbf{A}$ | $\mathbf{F}$ |  | Mr |  |
| Traffic Vol, veh/h | 56 | 229 | 163 | 44 | 90 | 114 |
| Future Vol, veh/h | 56 | 229 | 163 | 44 | 90 | 114 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 61 | 249 | 177 | 48 | 98 | 124 |


| Major/Minor | Major1 | Major2 |  |  | Minor2 |  |  |
| :--- | ---: | :--- | :--- | :--- | ---: | ---: | :---: |
| Conflicting Flow All | 225 | 0 | - | 0 | 572 | 201 |  |
| Stage 1 | - | - | - | - | 201 | - |  |
| $\quad$ Stage 2 | - | - | - | - | 371 | - |  |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |  |
| Follow-up Hdwy | 2.218 | - | - | -3.518 | 3.318 |  |  |
| Pot Cap-1 Maneuver | 1344 | - | - | - | 482 | 840 |  |
| $\quad$ Stage 1 | - | - | - | - | 833 | - |  |
| Stage 2 | - | - | - | - | 698 | - |  |
| Platoon blocked, \% |  | - | - | - |  |  |  |
| Mov Cap-1 Maneuver | 1344 | - | - | - | 456 | 840 |  |
| Mov Cap-2 Maneuver | - | - | - | - | 456 | - |  |
| Stage 1 | - | - | - | - | 789 | - |  |
| Stage 2 | - | - | - | - | 698 | - |  |


| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 1.5 | 0 | 14.2 |
| HCM LOS |  |  | B |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1344 | - | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.3 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\mathbf{7}$ |  |  | $\mathbf{\uparrow}$ | Tr |  |
| Traffic Vol, veh/h | 276 | 0 | 12 | 217 | 0 | 7 |
| Future Vol, veh/h | 276 | 0 | 12 | 217 | 0 | 7 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, $\#$ | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 300 | 0 | 13 | 236 | 0 | 8 |


| Major/Minor | Major1 | Major2 |  |  | Minor1 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | :---: |
| Conflicting Flow All | 0 | 0 | 300 | 0 | 562 | 300 |  |
| Stage 1 | - | - | - | - | 300 | - |  |
| Stage 2 | - | - | - | - | 262 | - |  |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |  |
| Follow-up Hdwy | - | -2.218 | -3.518 | 3.318 |  |  |  |
| Pot Cap-1 Maneuver | - | - | 1261 | - | 488 | 740 |  |
| $\quad$ Stage 1 | - | - | - | - | 752 | - |  |
| Stage 2 | - | - | - | - | 782 | - |  |
| Platoon blocked, \% | - | - |  | - |  |  |  |
| Mov Cap-1 Maneuver | - | - | 1261 | - | 482 | 740 |  |
| Mov Cap-2 Maneuver | - | - | - | - | 482 | - |  |
| Stage 1 | - | - | - | - | 752 | - |  |
| Stage 2 | - | - | - | - | 773 | - |  |


| Approach | EB | WB | NB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 0 | 0.4 | 9.9 |
| HCM LOS |  |  | A |


| Minor Lane/Major Mvmt | NBLn1 | EBT | EBR | WBL | WBT |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 740 | - | - | 1261 | - |
| HCM Lane V/C Ratio | 0.01 | - | - | 0.01 | - |
| HCM Control Delay (s) | 9.9 | - | - | 7.9 | 0 |
| HCM Lane LOS | A | - | - | A | A |
| HCM 95th \%tile Q(veh) | 0 | - | - | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 7.5 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | 1 | $\mathbf{7}$ | $\mathbf{1}$ | $\mathbf{4}$ | $\mathbf{F}$ |  |
| Traffic Vol, veh/h | 7 | 70 | 130 | 0 | 0 | 12 |
| Future Vol, veh/h | 7 | 70 | 130 | 0 | 0 | 12 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | 0 | 150 | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 8 | 76 | 141 | 0 | 0 | 13 |



|  | $\dagger$ | $\rightarrow$ | $\square$ |  |  | $\pm$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | ${ }^{7}$ | 4 | 4 | F | ${ }^{7}$ | 「 |
| Traffic Volume (vph) | 55 | 330 | 834 | 21 | 46 | 226 |
| Future Volume (vph) | 55 | 330 | 834 | 21 | 46 | 226 |
| Satd. Flow (prot) | 1770 | 1863 | 1863 | 1583 | 1770 | 1583 |
| Flt Permitted | 0.195 |  |  |  | 0.950 |  |
| Satd. Flow (perm) | 363 | 1863 | 1863 | 1583 | 1770 | 1583 |
| Satd. Flow (RTOR) |  |  |  | 23 |  | 218 |
| Lane Group Flow (vph) | 60 | 359 | 907 | 23 | 50 | 246 |
| Turn Type | pm+pt | NA | NA | Perm | Prot | Perm |
| Protected Phases | 5 | 2 | 6 |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  | 4 |
| Detector Phase | 5 | 2 | 6 | 6 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |
| Minimum Initial (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Minimum Split (s) | 10.0 | 25.0 | 25.0 | 25.0 | 25.0 | 25.0 |
| Total Split (s) | 10.0 | 75.0 | 65.0 | 65.0 | 25.0 | 25.0 |
| Total Split (\%) | 10.0\% | 75.0\% | 65.0\% | 65.0\% | 25.0\% | 25.0\% |
| Yellow Time (s) | 3.0 | 4.0 | 4.0 | 4.0 | 3.0 | 3.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 6.0 | 6.0 | 6.0 | 5.0 | 5.0 |
| Lead/Lag | Lead |  | Lag | Lag |  |  |
| Lead-Lag Optimize? | Yes |  | Yes | Yes |  |  |
| Recall Mode | None | C-Max | C-Max | C-Max | None | None |
| Act Effct Green (s) | 80.4 | 79.4 | 70.3 | 70.3 | 9.6 | 9.6 |
| Actuated g/C Ratio | 0.80 | 0.79 | 0.70 | 0.70 | 0.10 | 0.10 |
| v/c Ratio | 0.16 | 0.24 | 0.69 | 0.02 | 0.30 | 0.71 |
| Control Delay | 3.5 | 3.5 | 14.2 | 3.0 | 44.8 | 19.9 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 3.5 | 3.5 | 14.2 | 3.0 | 44.8 | 19.9 |
| LOS | A | A | B | A | D | B |
| Approach Delay |  | 3.5 | 13.9 |  | 24.1 |  |
| Approach LOS |  | A | B |  | C |  |
| Queue Length 50th (ft) | 5 | 41 | 299 | 0 | 31 | 17 |
| Queue Length 95th (ft) | 18 | 98 | 602 | 10 | 62 | 89 |
| Internal Link Dist (ft) |  | 1097 | 1210 |  | 815 |  |
| Turn Bay Length (ft) | 520 |  |  | 375 |  |  |
| Base Capacity (vph) | 379 | 1480 | 1309 | 1119 | 354 | 491 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.16 | 0.24 | 0.69 | 0.02 | 0.14 | 0.50 |
| Intersection Summary |  |  |  |  |  |  |
| Cycle Length: 100 |  |  |  |  |  |  |
| Actuated Cycle Length: 100 |  |  |  |  |  |  |
| Offset: $0(0 \%)$, Referenced to phase 2:EBTL and 6:WBT, Start of Green |  |  |  |  |  |  |
| Natural Cycle: 80 |  |  |  |  |  |  |
| Control Type: Actuated-Coordinated |  |  |  |  |  |  |

Maximum v/c Ratio: 0.71
Intersection Signal Delay: $13.1 \quad$ Intersection LOS: B

Intersection Capacity Utilization 67.1\% ICU Level of Service C Analysis Period (min) 15

Splits and Phases: 1: SH 60 \& High Plains Boulevard


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.1 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\boldsymbol{\uparrow}$ |  |  | $\neq 1$ |
| Traffic Vol, veh/h | 86 | 0 | 41 | 45 | 0 | 105 |
| Future Vol, veh/h | 86 | 0 | 41 | 45 | 0 | 105 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, $\#$ | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 93 | 0 | 45 | 49 | 0 | 114 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.7 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\mathbf{\uparrow}$ | $\mathbf{F}$ |  | M |  |
| Traffic Vol, veh/h | 48 | 127 | 241 | 47 | 77 | 60 |
| Future Vol, veh/h | 48 | 127 | 241 | 47 | 77 | 60 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 52 | 138 | 262 | 51 | 84 | 65 |


| Major/Minor M | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 313 | 0 | - | 0 | 530 | 288 |
| Stage 1 | - | - | - | - | 288 | - |
| Stage 2 | - | - | - | - | 242 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1247 | - | - | - | 510 | 751 |
| Stage 1 | - | - | - | - | 761 | - |
| Stage 2 | - | - | - | - | 798 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1247 | - | - | - | 487 | 751 |
| Mov Cap-2 Maneuver | - | - | - | - | 487 | - |
| Stage 1 | - | - | - | - | 727 | - |
| Stage 2 | - | - | - | - | 798 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 2.2 |  | 0 |  | 13.4 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT WBR SBLn1 |  |  |
| Capacity (veh/h) |  | 1247 | - | - | - | 576 |
| HCM Lane V/C Ratio |  | 0.042 | - | - | - | 0.259 |
| HCM Control Delay (s) |  | 8 | 0 | - | - | 13.4 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0.1 | - | - | - | 1 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.3 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\boldsymbol{F}$ |  |  | $\mathbf{4}$ | Mr |  |
| Traffic Vol, veh/h | 199 | 0 | 4 | 297 | 0 | 11 |
| Future Vol, veh/h | 199 | 0 | 4 | 297 | 0 | 11 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 216 | 0 | 4 | 323 | 0 | 12 |


| Major/Minor | Major1 | Major2 |  |  | Minor1 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | :---: |
| Conflicting Flow All | 0 | 0 | 216 | 0 | 547 | 216 |  |
| Stage 1 | - | - | - | - | 216 | - |  |
| Stage 2 | - | - | - | - | 331 | - |  |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |  |
| Follow-up Hdwy | - | - | 2.218 | -3.518 | 3.318 |  |  |
| Pot Cap-1 Maneuver | - | - | 1354 | - | 498 | 824 |  |
| $\quad$ Stage 1 | - | - | - | - | 820 | - |  |
| Stage 2 | - | - | - | - | 728 | - |  |
| Platoon blocked, \% | - | - |  | - |  |  |  |
| Mov Cap-1 Maneuver | - | - | 1354 | - | 496 | 824 |  |
| Mov Cap-2 Maneuver | - | - | - | - | 496 | - |  |
| Stage 1 | - | - | - | - | 820 | - |  |
| Stage 2 | - | - | - | - | 725 | - |  |


| Approach | EB | WB | NB |
| :--- | :---: | :---: | :---: |
| HCM Control Delay, s | 0 | 0.1 | 9.4 |
| HCM LOS |  |  | A |


| Minor Lane/Major Mvmt | NBLn1 | EBT | EBR | WBL | WBT |
| :--- | ---: | ---: | ---: | ---: | :---: |
| Capacity (veh/h) | 824 | - | -1354 | - |  |
| HCM Lane V/C Ratio | 0.015 | - | -0.003 | - |  |
| HCM Control Delay (s) | 9.4 | - | - | 7.7 | 0 |
| HCM Lane LOS | A | - | - | A | A |
| HCM 95th \%tile Q(veh) | 0 | - | - | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 8.1 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | i | $\mathbf{7}$ |  | $\mathbf{4}$ | $\mathbf{F}$ |  |
| Traffic Vol, veh/h | 11 | 105 | 41 | 0 | 0 | 4 |
| Future Vol, veh/h | 11 | 105 | 41 | 0 | 0 | 4 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | 0 | 150 | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 12 | 114 | 45 | 0 | 0 | 4 |



|  | 4 | $\rightarrow$ | $\downarrow$ |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | ${ }^{7}$ | 4 | 4 | 7 | ${ }^{1}$ | 「 |
| Traffic Volume (vph) | 251 | 765 | 452 | 74 | 56 | 114 |
| Future Volume (vph) | 251 | 765 | 452 | 74 | 56 | 114 |
| Satd. Flow (prot) | 1770 | 1863 | 1863 | 1583 | 1770 | 1583 |
| Flt Permitted | 0.420 |  |  |  | 0.950 |  |
| Satd. Flow (perm) | 782 | 1863 | 1863 | 1583 | 1770 | 1583 |
| Satd. Flow (RTOR) |  |  |  | 80 |  | 124 |
| Lane Group Flow (vph) | 273 | 832 | 491 | 80 | 61 | 124 |
| Turn Type | pm+pt | NA | NA | Perm | Prot | Perm |
| Protected Phases | 5 | 2 | 6 |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  | 4 |
| Detector Phase | 5 | 2 | 6 | 6 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |
| Minimum Initial (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Minimum Split (s) | 10.0 | 25.0 | 25.0 | 25.0 | 25.0 | 25.0 |
| Total Split (s) | 20.0 | 90.0 | 70.0 | 70.0 | 30.0 | 30.0 |
| Total Split (\%) | 16.7\% | 75.0\% | 58.3\% | 58.3\% | 25.0\% | 25.0\% |
| Yellow Time (s) | 3.0 | 4.0 | 4.0 | 4.0 | 3.0 | 3.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 6.0 | 6.0 | 6.0 | 5.0 | 5.0 |
| Lead/Lag | Lead |  | Lag | Lag |  |  |
| Lead-Lag Optimize? | Yes |  | Yes | Yes |  |  |
| Recall Mode | None | C-Max | C-Max | C-Max | None | None |
| Act Effct Green (s) | 100.5 | 99.5 | 85.3 | 85.3 | 9.5 | 9.5 |
| Actuated g/C Ratio | 0.84 | 0.83 | 0.71 | 0.71 | 0.08 | 0.08 |
| v/c Ratio | 0.37 | 0.54 | 0.37 | 0.07 | 0.44 | 0.52 |
| Control Delay | 3.6 | 4.9 | 8.4 | 1.7 | 61.5 | 16.8 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 3.6 | 4.9 | 8.4 | 1.7 | 61.5 | 16.8 |
| LOS | A | A | A | A | E | B |
| Approach Delay |  | 4.6 | 7.4 |  | 31.5 |  |
| Approach LOS |  | A | A |  | C |  |
| Queue Length 50th (ft) | 30 | 152 | 132 | 0 | 46 | 0 |
| Queue Length 95th (ft) | 58 | 263 | 228 | 17 | 89 | 57 |
| Internal Link Dist (ft) |  | 1097 | 1210 |  | 815 |  |
| Turn Bay Length (ft) | 520 |  |  | 375 |  |  |
| Base Capacity (vph) | 778 | 1544 | 1323 | 1147 | 368 | 427 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.35 | 0.54 | 0.37 | 0.07 | 0.17 | 0.29 |
| Intersection Summary |  |  |  |  |  |  |
| Cycle Length: 120 |  |  |  |  |  |  |
| Actuated Cycle Length: 120 |  |  |  |  |  |  |
| Offset: 0 (0\%), Referenced to phase 2:EBTL and 6:WBT, Start of Green |  |  |  |  |  |  |
| Natural Cycle: 60 |  |  |  |  |  |  |
| Control Type: Actuated-Coordinated |  |  |  |  |  |  |

Maximum v/c Ratio: 0.54
Intersection Signal Delay: $8.1 \quad$ Intersection LOS: A

Intersection Capacity Utilization 55.2\% ICU Level of Service B
Analysis Period (min) 15
Splits and Phases: 1: SH 60 \& High Plains Boulevard


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.1 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 68 | 2 | 129 | 92 | 2 | 68 |
| Future Vol, veh/h | 68 | 2 | 129 | 92 | 2 | 68 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 74 | 2 | 140 | 100 | 2 | 74 |


| Major/Minor M | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 268 | 190 | 0 | 0 | 240 | 0 |
| Stage 1 | 190 | - | - | - | - | - |
| Stage 2 | 78 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 |  | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 721 | 852 | - | - | 1327 | - |
| Stage 1 | 842 | - | - | - | - | - |
| Stage 2 | 945 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 720 | 852 | - | - | 1327 | - |
| Mov Cap-2 Maneuver | 720 | - | - | - | - | - |
| Stage 1 | 842 | - | - | - | - | - |
| Stage 2 | 943 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 10.6 |  | 0 |  | 0.2 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 723 | 1327 | - |
| HCM Lane V/C Ratio |  | - | - | 0.105 | 0.002 | - |
| HCM Control Delay (s) |  | - | - | 10.6 | 7.7 | 0 |
| HCM Lane LOS |  | - | - | B | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 0.4 | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 8.4 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\mathbf{A}$ | $\boldsymbol{\uparrow}$ |  | Mr |  |
| Traffic Vol, veh/h | 81 | 314 | 226 | 62 | 126 | 165 |
| Future Vol, veh/h | 81 | 314 | 226 | 62 | 126 | 165 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 88 | 341 | 246 | 67 | 137 | 179 |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 313 | 0 | - | 0 | 797 | 280 |
| Stage 1 | - | - | - | - | 280 | - |
| Stage 2 | - | - | - | - | 517 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1247 | - | - | - | 356 | 759 |
| Stage 1 | - | - | - | - | 767 | - |
| Stage 2 | - | - | - | - | 598 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1247 | - | - | - | 325 | 759 |
| Mov Cap-2 Maneuver | - | - | - | - | 325 | - |
| Stage 1 | - | - | - | - | 700 | - |
| Stage 2 | - | - | - | - | 598 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 1.7 |  | 0 |  | 25.7 |  |
| HCM LOS |  |  |  |  | D |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT WBR SBLn1 |  |  |
| Capacity (veh/h) |  | 1247 | - | - | - | 481 |
| HCM Lane V/C Ratio |  | 0.071 | - | - | - | 0.658 |
| HCM Control Delay (s) |  | 8.1 | 0 | - | - | 25.7 |
| HCM Lane LOS |  | A | A | - | - | D |
| HCM 95th \%tile Q(veh) |  | 0.2 | - | - | - | 4.7 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.2 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\boldsymbol{F}$ |  |  | $\mathbf{4}$ | Mr |  |
| Traffic Vol, veh/h | 398 | 0 | 12 | 300 | 0 | 7 |
| Future Vol, veh/h | 398 | 0 | 12 | 300 | 0 | 7 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 433 | 0 | 13 | 326 | 0 | 8 |


| Major/Minor | Major1 | Major2 |  |  | Minor1 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | :---: |
| Conflicting Flow All | 0 | 0 | 433 | 0 | 785 | 433 |  |
| Stage 1 | - | - | - | - | 433 | - |  |
| Stage 2 | - | - | - | - | 352 | - |  |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |  |
| Follow-up Hdwy | - | -2.218 | -3.518 | 3.318 |  |  |  |
| Pot Cap-1 Maneuver | - | - | 1127 | - | 361 | 623 |  |
| $\quad$ Stage 1 | - | - | - | - | 654 | - |  |
| Stage 2 | - | - | - | - | 712 | - |  |
| Platoon blocked, \% | - | - |  | - |  |  |  |
| Mov Cap-1 Maneuver | - | - | 1127 | - | 356 | 623 |  |
| Mov Cap-2 Maneuver | - | - | - | - | 356 | - |  |
| Stage 1 | - | - | - | - | 654 | - |  |
| Stage 2 | - | - | - | - | 702 | - |  |


| Approach | EB | WB | NB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 0 | 0.3 | 10.9 |
| HCM LOS |  |  | B |


| Minor Lane/Major Mvmt | NBLn1 | EBT | EBR | WBL | WBT |
| :--- | ---: | ---: | ---: | ---: | :---: |
| Capacity (veh/h) | 623 | - | -1127 | - |  |
| HCM Lane V/C Ratio | 0.012 | - | -0.012 | - |  |
| HCM Control Delay (s) | 10.9 | - | - | 8.2 | 0 |
| HCM Lane LOS | B | - | - | A | A |
| HCM 95th \%tile Q(veh) | 0 | - | - | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 7.5 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | 1 | $\mathbf{7}$ | $\mathbf{1}$ | $\mathbf{4}$ | $\mathbf{F}$ |  |
| Traffic Vol, veh/h | 7 | 70 | 131 | 0 | 0 | 12 |
| Future Vol, veh/h | 7 | 70 | 131 | 0 | 0 | 12 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | 0 | 150 | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 8 | 76 | 142 | 0 | 0 | 13 |




| Major/Minor | Major1 | Major2 |  | Minor2 |  |  |
| :--- | ---: | :--- | :--- | :--- | ---: | ---: |
| Conflicting Flow All | 656 | 0 | - | 0 | 1095 | 628 |
| Stage 1 | - | - | - | - | 628 | - |
| $\quad$ Stage 2 | - | - | - | - | 467 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | -3.518 | 3.318 |  |
| Pot Cap-1 Maneuver | 931 | - | - | - | 236 | 483 |
| $\quad$ Stage 1 | - | - | - | - | 532 | - |
| Stage 2 | - | - | - | - | 631 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 931 | - | - | - | 208 | 483 |
| Mov Cap-2 Maneuver | - | - | - | - | 208 | - |
| Stage 1 | - | - | - | - | 470 | - |
| Stage 2 | - | - | - | - | 631 | - |


| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 2.9 | 0 | 31.9 |
| HCM LOS |  |  | D |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 931 | - | - | - | 208 | 483 |
| HCM Lane V/C Ratio | 0.117 | - | - | -0.334 | 0.756 |  |
| HCM Control Delay (s) | 9.4 | - | - | - | 30.8 | 32.1 |
| HCM Lane LOS | A | - | - | - | D | D |
| HCM 95th \%tile Q(veh) | 0.4 | - | - | - | 1.4 | 6.5 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.2 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\uparrow$ |  |  | $\neq 1$ |
| Traffic Vol, veh/h | 86 | 0 | 92 | 41 | 0 | 257 |
| Future Vol, veh/h | 86 | 0 | 92 | 41 | 0 | 257 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 93 | 0 | 100 | 45 | 0 | 279 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.8 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\mathbf{A}$ | $\mathbf{F}$ |  | Mr |  |
| Traffic Vol, veh/h | 33 | 110 | 232 | 49 | 58 | 42 |
| Future Vol, veh/h | 33 | 110 | 232 | 49 | 58 | 42 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 36 | 120 | 252 | 53 | 63 | 46 |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 305 | 0 | - | 0 | 471 | 279 |
| Stage 1 | - | - | - | - | 279 | - |
| Stage 2 | - | - | - | - | 192 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1256 | - | - | - | 551 | 760 |
| Stage 1 | - | - | - | - | 768 | - |
| Stage 2 | - | - | - | - | 841 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1256 | - | - | - | 534 | 760 |
| Mov Cap-2 Maneuver | - | - | - | - | 534 | - |
| Stage 1 | - | - | - | - | 744 | - |
| Stage 2 | - | - | - | - | 841 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 1.8 |  | 0 |  | 12.2 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT WBR SBLn1 |  |  |
| Capacity (veh/h) |  | 1256 | - | - | - | 610 |
| HCM Lane V/C Ratio |  | 0.029 | - | - | - | 0.178 |
| HCM Control Delay (s) |  | 8 | 0 | - | - | 12.2 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0.1 | - | - |  | 0.6 |


|  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Intersection |  |  |  |  |  |  |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\uparrow$ |  |  | ¢ | Fr |  |
| Traffic Vol, veh/h | 153 | 9 | 32 | 262 | 28 | 20 |
| Future Vol, veh/h | 153 | 9 | 32 | 262 | 28 | 20 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 166 | 10 | 35 | 285 | 30 | 22 |


| Major/Minor M | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 176 | 0 | 526 | 171 |
| Stage 1 | - | - | - | - | 171 | - |
| Stage 2 | - | - | - | - | 355 | - |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | - | - | 2.218 | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | - | - | 1400 | - | 512 | 873 |
| Stage 1 | - | - | - | - | 859 | - |
| Stage 2 | - | - | - | - | 710 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1400 | - | 497 | 873 |
| Mov Cap-2 Maneuver | - | - | - | - | 497 | - |
| Stage 1 | - | - | - | - | 859 | - |
| Stage 2 | - | - | - | - | 689 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | NB |  |
| HCM Control Delay, s | 0 |  | 0.8 |  | 11.5 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBT | EBR | WBL WBT |  |
| Capacity (veh/h) |  | 606 | - | - | 1400 | - |
| HCM Lane V/C Ratio |  | 0.086 | - | - | 0.025 | - |
| HCM Control Delay (s) |  | 11.5 | - | - | 7.6 | 0 |
| HCM Lane LOS |  | B | - | - | A | A |
| HCM 95th \%tile Q(veh) |  | 0.3 | - | - | 0.1 | - |


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.4 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\boldsymbol{\beta}$ |  |  | $\mathbf{\epsilon}$ | Mr |  |
| Traffic Vol, veh/h | 150 | 23 | 10 | 225 | 69 | 28 |
| Future Vol, veh/h | 150 | 23 | 10 | 225 | 69 | 28 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 163 | 25 | 11 | 245 | 75 | 30 |



|  | EB | WB | NB |
| :--- | ---: | ---: | ---: |
| Approach | CCM Control Delay, s | 0 | 0.3 |
| HCM LOS |  | 11.9 |  |
| B |  |  |  |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 13.9 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | ${ }^{*}$ | 4 | 4 | 「 | ${ }^{7}$ | 「 |
| Traffic Vol, veh/h 37 | 378 | 530 | 313 | 88 | 61 | 196 |
| Future Vol, veh/h | 378 | 530 | 313 | 88 | 61 | 196 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control Fr | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 520 | - | - | 375 | 0 | 0 |
| Veh in Median Storage, \# | \# - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 411 | 576 | 340 | 96 | 66 | 213 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.4 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | T |  | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 65 | 1 | 300 | 94 | 1 | 170 |
| Future Vol, veh/h | 65 | 1 | 300 | 94 | 1 | 170 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 71 | 1 | 326 | 102 | 1 | 185 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 5.2 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\mathbf{\uparrow}$ | $\mathbf{F}$ |  | $\mathbf{F}$ |  |
| Traffic Vol, veh/h | 56 | 291 | 199 | 53 | 105 | 114 |
| Future Vol, veh/h | 56 | 291 | 199 | 53 | 105 | 114 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 61 | 316 | 216 | 58 | 114 | 124 |


| Major/Minor | Major1 | Major2 |  | Minor2 |  |  |
| :---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Conflicting Flow All | 274 | 0 | - | 0 | 683 | 245 |
| Stage 1 | - | - | - | - | 245 | - |
| Stage 2 | - | - | - | - | 438 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | -3.518 | 3.318 |  |
| Pot Cap-1 Maneuver | 1289 | - | - | - | 415 | 794 |
| Stage 1 | - | - | - | - | 796 | - |
| Stage 2 | - | - | - | - | 651 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1289 | - | - | - | 391 | 794 |
| Mov Cap-2 Maneuver | - | - | - | - | 391 | - |
| Stage 1 | - | - | - | - | 751 | - |
| Stage 2 | - | - | - | - | 651 | - |


| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 1.3 | 0 | 17.2 |
| HCM LOS |  |  | C |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1289 | - | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.4 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\boldsymbol{F}$ |  |  | $\mathbf{4}$ | Mr |  |
| Traffic Vol, veh/h | 323 | 30 | 31 | 244 | 18 | 38 |
| Future Vol, veh/h | 323 | 30 | 31 | 244 | 18 | 38 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 351 | 33 | 34 | 265 | 20 | 41 |





| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.7 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\boldsymbol{F}$ |  |  | $\mathbf{4}$ | Mr |  |
| Traffic Vol, veh/h | 283 | 78 | 31 | 229 | 46 | 18 |
| Future Vol, veh/h | 283 | 78 | 31 | 229 | 46 | 18 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 308 | 85 | 34 | 249 | 50 | 20 |


| Major/Minor | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 393 | 0 | 668 | 351 |
| Stage 1 | - | - | - | - | 351 | - |
| Stage 2 | - | - | - | - | 317 | - |
| Critical Hdwy | - | - | 4.12 |  | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | - | - | 2.218 | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | - | - | 1166 | - | 423 | 692 |
| Stage 1 | - | - | - | - | 713 | - |
| Stage 2 | - | - | - | - | 738 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1166 | - | 409 | 692 |
| Mov Cap-2 Maneuver | - | - | - | - | 409 | - |
| Stage 1 | - | - | - | - | 713 | - |
| Stage 2 | - | - | - | - | 713 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | NB |  |
| HCM Control Delay, s | 0 |  | 1 |  | 14.2 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBT | EBR | WBL WBT |  |
| Capacity (veh/h) |  | 462 | - | - | 1166 | - |
| HCM Lane V/C Ratio |  | 0.151 | - |  | 0.029 | - |
| HCM Control Delay (s) |  | 14.2 | - | - | 8.2 | 0 |
| HCM Lane LOS |  | B | - | - | A | A |
| HCM 95th \%tile Q(veh) |  | 0.5 | - | - | 0.1 | - |


|  | 4 | $\rightarrow$ | $\nsim$ |  |  | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | ${ }^{7}$ | 4 | 4 | 7 | ${ }^{7}$ | 「 |
| Traffic Volume (vph) | 107 | 330 | 834 | 31 | 73 | 378 |
| Future Volume (vph) | 107 | 330 | 834 | 31 | 73 | 378 |
| Satd. Flow (prot) | 1770 | 1863 | 1863 | 1583 | 1770 | 1583 |
| Flt Permitted | 0.140 |  |  |  | 0.950 |  |
| Satd. Flow (perm) | 261 | 1863 | 1863 | 1583 | 1770 | 1583 |
| Satd. Flow (RTOR) |  |  |  | 34 |  | 218 |
| Lane Group Flow (vph) | 116 | 359 | 907 | 34 | 79 | 411 |
| Turn Type | pm+pt | NA | NA | Perm | Prot | Perm |
| Protected Phases | 5 | 2 | 6 |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  | 4 |
| Detector Phase | 5 | 2 | 6 | 6 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |
| Minimum Initial (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Minimum Split (s) | 10.0 | 25.0 | 25.0 | 25.0 | 25.0 | 25.0 |
| Total Split (s) | 10.0 | 75.0 | 65.0 | 65.0 | 25.0 | 25.0 |
| Total Split (\%) | 10.0\% | 75.0\% | 65.0\% | 65.0\% | 25.0\% | 25.0\% |
| Yellow Time (s) | 3.0 | 4.0 | 4.0 | 4.0 | 3.0 | 3.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 6.0 | 6.0 | 6.0 | 5.0 | 5.0 |
| Lead/Lag | Lead |  | Lag | Lag |  |  |
| Lead-Lag Optimize? | Yes |  | Yes | Yes |  |  |
| Recall Mode | None | C-Max | Max | Max | None | None |
| Act Effct Green (s) | 72.9 | 71.9 | 61.4 | 61.4 | 17.1 | 17.1 |
| Actuated g/C Ratio | 0.73 | 0.72 | 0.61 | 0.61 | 0.17 | 0.17 |
| v/c Ratio | 0.42 | 0.27 | 0.79 | 0.03 | 0.26 | 0.91 |
| Control Delay | 9.4 | 6.0 | 22.0 | 3.1 | 36.8 | 44.0 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 9.4 | 6.0 | 22.0 | 3.1 | 36.8 | 44.0 |
| LOS | A | A | C | A | D | D |
| Approach Delay |  | 6.8 | 21.3 |  | 42.9 |  |
| Approach LOS |  | A | C |  | D |  |
| Queue Length 50th (ft) | 21 | 77 | 430 | 0 | 43 | 122 |
| Queue Length 95th (ft) | 38 | 116 | 630 | 12 | 84 | \#287 |
| Internal Link Dist (ft) |  | 1097 | 1210 |  | 815 |  |
| Turn Bay Length (ft) | 520 |  |  | 375 |  |  |
| Base Capacity (vph) | 273 | 1338 | 1143 | 984 | 354 | 491 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.42 | 0.27 | 0.79 | 0.03 | 0.22 | 0.84 |
| Intersection Summary |  |  |  |  |  |  |
| Cycle Length: 100 |  |  |  |  |  |  |
| Actuated Cycle Length: 100 |  |  |  |  |  |  |
| Offset: 0 (0\%), Referenced to phase 2:EBTL, Start of Green |  |  |  |  |  |  |
| Natural Cycle: 80 |  |  |  |  |  |  |
| Control Type: Actuated-Coordinated |  |  |  |  |  |  |

Maximum v/c Ratio: 0.91
Intersection Signal Delay: 23.3 Intersection LOS: C
Intersection Capacity Utilization 76.5\% ICU Level of Service D

Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: SH 60 \& High Plains Boulevard


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.7 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\uparrow$ |  |  | $\neq 1$ |
| Traffic Vol, veh/h | 113 | 0 | 93 | 55 | 0 | 257 |
| Future Vol, veh/h | 113 | 0 | 93 | 55 | 0 | 257 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 123 | 0 | 101 | 60 | 0 | 279 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.6 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\mathbf{A}$ | $\boldsymbol{\uparrow}$ |  | Mr |  |
| Traffic Vol, veh/h | 48 | 146 | 296 | 61 | 81 | 60 |
| Future Vol, veh/h | 48 | 146 | 296 | 61 | 81 | 60 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, $\#$ | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 52 | 159 | 322 | 66 | 88 | 65 |


| Major/Minor | Major1 | Major2 |  | Minor2 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Conflicting Flow All | 388 | 0 | - | 0 | 618 | 355 |
| $\quad$ Stage 1 | - | - | - | - | 355 | - |
| $\quad$ Stage 2 | - | - | - | - | 263 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1170 | - | - | - | 453 | 689 |
| $\quad$ Stage 1 | - | - | - | - | 710 | - |
| $\quad$ Stage 2 | - | - | - | - | 781 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1170 | - | - | - | 431 | 689 |
| Mov Cap-2 Maneuver | - | - | - | - | 431 | - |
| Stage 1 | - | - | - | - | 675 | - |
| Stage 2 | - | - | - | - | 781 | - |
|  |  |  |  |  |  |  |


| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 2 | 0 | 15 |
| HCM LOS |  | $C$ |  |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1170 | - | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.4 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\uparrow$ |  |  | et | M |  |
| Traffic Vol, veh/h | 213 | 9 | 32 | 338 | 28 | 20 |
| Future Vol, veh/h | 213 | 9 | 32 | 338 | 28 | 20 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 232 | 10 | 35 | 367 | 30 | 22 |


| Major/Minor M | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 242 | 0 | 674 | 237 |
| Stage 1 | - | - | - | - | 237 | - |
| Stage 2 | - | - | - | - | 437 | - |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | - | - | 2.218 | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | - | - | 1324 | - | 420 | 802 |
| Stage 1 | - | - | - | - | 802 | - |
| Stage 2 | - | - | - | - | 651 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1324 | - | 406 | 802 |
| Mov Cap-2 Maneuver | - | - | - | - | 406 | - |
| Stage 1 | - | - | - | - | 802 | - |
| Stage 2 | - | - | - | - | 630 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | NB |  |
| HCM Control Delay, s | 0 |  | 0.7 |  | 12.8 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 EBT EBR WBL WBT |  |  |  |  |
| Capacity (veh/h) |  | 511 | - | - | 1324 | - |
| HCM Lane V/C Ratio |  | 0.102 | - | - | 0.026 | - |
| HCM Control Delay (s) |  | 12.8 | - | - | 7.8 | 0 |
| HCM Lane LOS |  | B | - | - | A | A |
| HCM 95th \%tile Q(veh) |  | 0.3 | - | - | 0.1 | - |


| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.1 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\boldsymbol{F}$ |  |  | $\mathbf{-}$ | Mr |  |
| Traffic Vol, veh/h | 210 | 23 | 10 | 301 | 69 | 28 |
| Future Vol, veh/h | 210 | 23 | 10 | 301 | 69 | 28 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 228 | 25 | 11 | 327 | 75 | 30 |


| Major/Minor | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 253 | 0 | 590 | 241 |
| Stage 1 | - | - | - | - | 241 | - |
| Stage 2 | - | - | - | - | 349 | - |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - |  | - | 5.42 | - |
| Follow-up Hdwy | - | - | 2.218 | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | - | - | 1312 | - | 470 | 798 |
| Stage 1 | - | - | - | - | 799 | - |
| Stage 2 | - | - | - | - | 714 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1312 | - | 465 | 798 |
| Mov Cap-2 Maneuver | - | - | - | - | 465 | - |
| Stage 1 | - | - | - | - | 799 | - |
| Stage 2 | - | - | - | - | 707 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | NB |  |
| HCM Control Delay, s | 0 |  | 0.2 |  | 13.5 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBT | EBR | WBL WBT |  |
| Capacity (veh/h) |  | 529 | - | - | 1312 | - |
| HCM Lane V/C Ratio |  | 0.199 | - |  | 0.008 | - |
| HCM Control Delay (s) |  | 13.5 | - | - | 7.8 | 0 |
| HCM Lane LOS |  | B | - | - | A | A |
| HCM 95th \%tile Q(veh) |  | 0.7 | - | - | 0 | - |


|  | 4 | $\rightarrow$ |  | $4$ |  | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | ${ }^{7}$ | 4 | 4 | F | ${ }^{*}$ | 「 |
| Traffic Volume (vph) | 422 | 765 | 452 | 105 | 75 | 214 |
| Future Volume (vph) | 422 | 765 | 452 | 105 | 75 | 214 |
| Satd. Flow (prot) | 1770 | 1863 | 1863 | 1583 | 1770 | 1583 |
| Flt Permitted | 0.403 |  |  |  | 0.950 |  |
| Satd. Flow (perm) | 751 | 1863 | 1863 | 1583 | 1770 | 1583 |
| Satd. Flow (RTOR) |  |  |  | 114 |  | 233 |
| Lane Group Flow (vph) | 459 | 832 | 491 | 114 | 82 | 233 |
| Turn Type | pm+pt | NA | NA | Perm | Prot | Perm |
| Protected Phases | 5 | 2 | 6 |  | 4 |  |
| Permitted Phases | 2 |  |  | 6 |  | 4 |
| Detector Phase | 5 | 2 | 6 | 6 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |
| Minimum Initial (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Minimum Split (s) | 10.0 | 25.0 | 25.0 | 25.0 | 25.0 | 25.0 |
| Total Split (s) | 20.0 | 90.0 | 70.0 | 70.0 | 30.0 | 30.0 |
| Total Split (\%) | 16.7\% | 75.0\% | 58.3\% | 58.3\% | 25.0\% | 25.0\% |
| Yellow Time (s) | 3.0 | 4.0 | 4.0 | 4.0 | 3.0 | 3.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 6.0 | 6.0 | 6.0 | 5.0 | 5.0 |
| Lead/Lag | Lead |  | Lag | Lag |  |  |
| Lead-Lag Optimize? | Yes |  | Yes | Yes |  |  |
| Recall Mode | None | C-Max | Max | Max | None | None |
| Act Effct Green (s) | 99.1 | 98.1 | 79.6 | 79.6 | 10.9 | 10.9 |
| Actuated g/C Ratio | 0.83 | 0.82 | 0.66 | 0.66 | 0.09 | 0.09 |
| v/c Ratio | 0.63 | 0.55 | 0.40 | 0.10 | 0.51 | 0.66 |
| Control Delay | 6.8 | 5.5 | 11.5 | 2.1 | 62.4 | 15.6 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 6.8 | 5.5 | 11.5 | 2.1 | 62.4 | 15.6 |
| LOS | A | A | B | A | E | B |
| Approach Delay |  | 6.0 | 9.7 |  | 27.8 |  |
| Approach LOS |  | A | A |  | C |  |
| Queue Length 50th (ft) | 64 | 166 | 158 | 0 | 62 | 0 |
| Queue Length 95th (ft) | 118 | 293 | 285 | 24 | 111 | 75 |
| Internal Link Dist (ft) |  | 1097 | 1210 |  | 815 |  |
| Turn Bay Length (ft) | 520 |  |  | 375 |  |  |
| Base Capacity (vph) | 754 | 1523 | 1236 | 1088 | 368 | 514 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.61 | 0.55 | 0.40 | 0.10 | 0.22 | 0.45 |
| Intersection Summary |  |  |  |  |  |  |
| Cycle Length: 120 |  |  |  |  |  |  |
| Actuated Cycle Length: 120 |  |  |  |  |  |  |
| Offset: 0 (0\%), Referenced to phase 2:EBTL, Start of Green |  |  |  |  |  |  |
| Natural Cycle: 75 |  |  |  |  |  |  |
| Control Type: Actuated-Coordinated |  |  |  |  |  |  |

Maximum v/c Ratio: 0.66
Intersection Signal Delay: 10.1 Intersection LOS: B

Intersection Capacity Utilization 64.7\% ICU Level of Service C Analysis Period (min) 15

Splits and Phases: 1: SH 60 \& High Plains Boulevard


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.9 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 86 | 2 | 300 | 123 | 2 | 169 |
| Future Vol, veh/h | 86 | 2 | 300 | 123 | 2 | 169 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 93 | 2 | 326 | 134 | 2 | 184 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 12.1 |  |  |  |  |  |
| Movement E | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | ${ }_{1} 1$ | $\uparrow$ |  | M |  |
| Traffic Vol, veh/h | 81 | 376 | 262 | 71 | 141 | 165 |
| Future Vol, veh/h | 81 | 376 | 262 | 71 | 141 | 165 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control Fr | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | \# | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 88 | 409 | 285 | 77 | 153 | 179 |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 362 | 0 | - | 0 | 909 | 324 |
| Stage 1 | - | - | - | - | 324 | - |
| Stage 2 | - | - | - | - | 585 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1197 | - | - | - | 305 | 717 |
| Stage 1 | - | - | - | - | 733 | - |
| Stage 2 | - | - | - | - | 557 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1197 | - | - | - | 276 | 717 |
| Mov Cap-2 Maneuver | - | - | - | - | 276 | - |
| Stage 1 | - | - | - | - | 663 | - |
| Stage 2 | - | - | - | - | 557 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 1.5 |  | 0 |  | 41.2 |  |
| HCM LOS |  |  |  |  | E |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 |  |
| Capacity (veh/h) |  | 1197 | - | - | - | 413 |
| HCM Lane V/C Ratio |  | 0.074 | - | - | - | 0.805 |
| HCM Control Delay (s) |  | 8.2 | 0 | - | - | 41.2 |
| HCM Lane LOS |  | A | A | - | - | E |
| HCM 95th \%tile Q(veh) |  | 0.2 | - | - | - | 7.2 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.2 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\boldsymbol{F}$ |  |  | $\mathbf{4}$ | Mr |  |
| Traffic Vol, veh/h | 445 | 30 | 31 | 327 | 18 | 38 |
| Future Vol, veh/h | 445 | 30 | 31 | 327 | 18 | 38 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 484 | 33 | 34 | 355 | 20 | 41 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\boldsymbol{F}$ |  |  | $\mathbf{4}$ | Mr |  |
| Traffic Vol, veh/h | 405 | 78 | 31 | 312 | 46 | 18 |
| Future Vol, veh/h | 405 | 78 | 31 | 312 | 46 | 18 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 440 | 85 | 34 | 339 | 50 | 20 |


| Major/Minor | Major1 | Major2 |  |  | Minor1 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | :---: |
| Conflicting Flow All | 0 | 0 | 525 | 0 | 890 | 483 |  |
| Stage 1 | - | - | - | - | 483 | - |  |
| Stage 2 | - | - | - | - | 407 | - |  |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |  |
| Follow-up Hdwy | - | -2.218 | -3.518 | 3.318 |  |  |  |
| Pot Cap-1 Maneuver | - | - | 1042 | - | 313 | 584 |  |
| $\quad$ Stage 1 | - | - | - | - | 620 | - |  |
| Stage 2 | - | - | - | - | 672 | - |  |
| Platoon blocked, \% | - | - |  | - |  |  |  |
| Mov Cap-1 Maneuver | - | - | 1042 | - | 300 | 584 |  |
| Mov Cap-2 Maneuver | - | - | - | - | 300 | - |  |
| Stage 1 | - | - | - | - | 620 | - |  |
| Stage 2 | - | - | - | - | 645 | - |  |


| Approach | EB | WB | NB |
| :--- | ---: | :---: | :---: |
| HCM Control Delay, s | 0 | 0.8 | 17.9 |
| HCM LOS |  |  | C |


| Minor Lane/Major Mvmt | NBLn1 | EBT | EBR | WBL | WBT |
| :--- | ---: | ---: | ---: | ---: | :---: |
| Capacity (veh/h) | 348 | - | -1042 | - |  |
| HCM Lane V/C Ratio | 0.2 | - | -0.032 | - |  |
| HCM Control Delay (s) | 17.9 | - | - | 8.6 | 0 |
| HCM Lane LOS | C | - | - | A | A |
| HCM 95th \%tile Q(veh) | 0.7 | - | - | 0.1 | - |

## APPENDIX D

Warrant Analysis Forms

Figure 4C-3. Warrant 3, Peak Hour

*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Figure 4C-4. Warrant 3, Peak Hour (70\% Factor) (COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)

*Note: 100 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold volume for a minor-street approach with one lane.

Figure 4C-3. Warrant 3, Peak Hour

*Note: 150 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 100 vph applies as the lower threshold volume for a minor-street approach with one lane.

Figure 4C-4. Warrant 3, Peak Hour (70\% Factor) (COMMUNITY LESS THAN 10,000 POPULATION OR ABOVE 40 MPH ON MAJOR STREET)

*Note: 100 vph applies as the lower threshold volume for a minor-street approach with two or more lanes and 75 vph applies as the lower threshold volume for a minor-street approach with one lane.

## Town of Johnstown

# Blue Sky Prairie 

 Preliminary Plat \& Development PlanPreliminary Plat \& Development Plan (SUB23-0008)

Planning \& Zoning Commission October 18, 2023

## Location:

- Located South of Veteran's Parkway and East of High Plains Blvd
- Approx. 121 Acres


## Background:

- 2004- Klein 125 Annexation
- 2023- High Plains Estates ODP ~ Amendment (Supersedes Klein PUD)



## SUBDIVISION

- Estimated maximum ~ 413 lots
- Mix of row homes, paired homes \& clustered single- family
- (3,000-6,000 SF) lots
- Outlots will be maintained by the Metro District
- Unique walkways in subdivision to create an open feel to the neighborhood



## DEVELOPMENT PLAN

## Community Amenities

- Expanded Trail network that connects the outlots and pocket park/amenity center for the community along ROW, 3 pocket parks


## Housing Diversity

| HIGH PLAINS ESTATES -PRELIMINARY LOT MIX |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Typical Lot Frontage (Per ODP) | Lot Size | Home Type | Number of Dwelling units Minimum | Number of Dwelling units Maximum | Estimated Number of Dwelling Units | PDP <br> Number of <br> Dwelling <br> Units |
| 60-feet | 6,000 SF and greater | SFD | 50 | 120 | 94 | 116 |
| 50-feet | 5,000 SF | SFD | 70 | 120 | 107 | 67 |
| 40-feet | 4,000 SF | SFD | 80 | 150 | 131 | 100 |
| 35-feet | 3,000 SF | SFD | 50 | 150 | 121 | 130 |
| SUBTOTAL |  |  |  |  |  | 413 |

## Phasing




| 60' SINGLE FAMIY DETACHED |
| :---: |
| 50' |
| 40' SINGLE FAMILY Detached |
| 35' REAR-LOAD SINGLE FAMILY DETACHED |
| $35^{\prime}$ FRONT-LOAD SINGLE FAMILY DETACHED |

PLANNING AREA 1: $53 \mathrm{DU} / 12.3 \mathrm{DEV} . \mathrm{AC}=4.31 \mathrm{DU} / \mathrm{AC}$
PLANNING AREA 2: $128 \mathrm{DU} / 28.1 \mathrm{AC}=4.56 \mathrm{DU} / \mathrm{AC}$
PLANNING AREA 3: $81 \mathrm{DU} / 28.5 \mathrm{AC}=2.84 \mathrm{DU} / \mathrm{AC}$
PLANNING AREA 4: 151 DU $/ 30.2 \mathrm{AC}=5.00 \mathrm{DU} / \mathrm{AC}$

## ANALYSIS \& RECOMMENDATION

- Conforms to the High Plains Village Amended (ODP) .
- In substantial compliance with the Town's codes, regulations, and requirements
- Promotes the Towns goals of diversity of housing types, unique neighborhood creation, and an efficient pattern of development.
- Staff Recommendation for Approval


## Town of Johnstown

## PLANNING \& ZONING COMMISSION AGENDA MEMORANDUM

## ITEM: Massey Square Change of Zone from PUD-B to MU-NC (ZON23-0001)

DESCRIPTION: Change of zone for 21.1 acres within the Johnstown Village P.U.D. from PUD-B (Commercial/Office) to MU-NC (Neighborhood Commercial)

LOCATION:
SE corner of Highway 60 and WCR 13/Colorado Blvd.
APPLICANT: Massey Farms, LLLP
STAFF: Tony LeFevre, Planner I
HEARING DATE: November 1,2023

## ATTACHMENTS

1- Vicinity Map
2- Zoning Map

## BACKGROUND AND SUMMARY

The applicant/owner, Massey Farms LLLP, request a change of zoning for approximately 21.2 acres of land, located east of WCR 13/Colorado Blvd. and south of Highway 60.

The subject property was annexed into the Town in 2006 as part of Massey Farms 141 Annexation and was zoned PUD-MU, with a designated land use of Commercial/Office over most of the current acreage. The subject property was part of the 2019 Johnstown Village P.U.D. Amendment 1 Final Development Plan where it maintained a Commercial land use designation. Historically, the property has been used for agricultural use.

## SURROUNDING ZONING \& LAND USE

| North: | PUD-Commercial/Retail (Purvis PUD) and Unincorporated Weld County |
| :--- | :--- |
| East: | PUD-R - Johnstown Village Filing 3 (Johnstown Village Townhomes) |
| South: | PUD-R - Johnstown Village Filing 2 (Pintail Commons Duplexes) |
| West: | PUD-MU - Clearview PUD Commercial/Retail- undeveloped |

## PUBLIC NOTICE AND AGENCY REFERRALS

Notice for the Planning and Zoning Commission hearing was published in the local paper of widest circulation, the Johnstown Breeze, on Thursday, October 26, 2023. This notice provided the date, time, and location of the Planning and Zoning Commission hearing, as well as a description of the project.

Notices were mailed to all property owners within 800 feet of the property in questions. This notice included a map of the proposed zoning changes.

Based upon the changes proposed, this land use designation amendment did not warrant a full referral review by the Johnstown Review Committee (JRC). Submitted documents were provided to the JRC for a preliminary evaluation. No concerns or comments by the JRC were noted.

## NEIGHBORHOOD MEETING

No neighborhood meeting was held for this requested zoning change as the MU-NC zone provides a similar range of uses to the PUD-B zoning in the original Johnstown Village PUD. The Town will hold neighborhood meetings for the surrounding property owners with the future development plans, when more detailed uses and plans are known.

## STAFF ANALYSIS

This property is located along a main activity corridor for the Town, with expectations of medium intensity and development. Both Highway 60 and Colorado Blvd. are major arterials and updates to existing roadway infrastructure are planned to accommodate this level of development, with right of way dedications and road improvements for this and surrounding properties. The zone change to MU-NC allows for the Town and developers to work off of the updated 2023 Town of Johnstown Land Use Code, and this Neighborhood Commercial designation is compatible with the Comprehensive Plan.

## RECOMMENDED PLANNING AND ZONING COMMISSION FINDINGS AND MOTIONS

It is recommended that Planning and Zoning Commission send a positive recommendation to Town Council that the requested zoning of MU-NC for Massey Square be approved based upon the finding that the proposed zoning will allow appropriate levels of development to occur in alignment with the Town's Comprehensive Plan.

## Recommended Motion

Based on the application materials received and analysis and presentation at the hearing, the Planning \& Zoning Commission finds that the request for MU-NC for Massey Square furthers the Johnstown Area Comprehensive Plan, and is compatible with surrounding neighborhoods, and therefore moves to recommend to the Town Council approval of the request for PUD-MU zoning for this area based upon the finding as stated above.

## Alternate Motions

Motion to Deny: "I move that the Commission recommend to the Town Council denial of the MU-NC zoning for Massey Square based upon the following...."



## Town of Johnstown

## PLANNING \& ZONING COMMISSION AGENDA MEMORANDUM

ITEM: Vista Commons Change of Zone from PUD-MU to MU-RC \& R3 (ZON23-0002)

DESCRIPTION: Change of zone for 147.4 acres from the Vista Commons P.U.D. from PUD-MU (Commercial/Industrial) to MU-RC (Regional Commercial) and R3 (Residential)

LOCATION: NW corner of Highway 60 and High Plains Blvd.
APPLICANT: I-25 Johnstown Partners, LLC
STAFF: Tony LeFevre, Planner I
HEARING DATE: November 1, 2023

## ATTACHMENTS

1- Vicinity Map
2- Zoning Map

## BACKGROUND AND SUMMARY

The applicant I-25 Johnstown Partners, LLC is requesting a change of zoning for approximately 147.4 acres of land, located west of High Plains Blvd. and north of Highway 60.

The subject property was annexed into the Town in 2000 as the Pratt Technology Campus and was zoned PUD-MU, with a designated land use of Commercial \& Industrial over the acreage. Historically, the property has been used for agricultural use.

## SURROUNDING ZONING \& LAND USE

North: PUD-MU Great Plains Village - Residential and Mixed Use/Employment (Revere)
East: $\quad$ R1 - Roxbury Ridge Residential
South: PUD-MU - Ledge Rock Center
West: PUD-MU - I-25 Gateway Center (Retail/Commercial/Light Industrial)

## PUBLIC NOTICE AND AGENCY REFERRALS

Notice for the Planning and Zoning Commission hearing was published in the local paper of widest circulation, the Johnstown Breeze, on Thursday, October 5, 2023. This notice provided the date, time, and location of the Planning and Zoning Commission hearing, as well as a description of the project. Notices were mailed to all property owners within 800 feet of the property in questions. This notice included a map of the proposed zoning changes.

Based upon the changes proposed, this land use designation amendment did not warrant a full referral review by the Johnstown Review Committee (JRC). Submitted documents were provided to the JRC for a preliminary evaluation. No concerns or comments by the JRC were noted specifically related to the change of zoning of the subject property.

## NEIGHBORHOOD MEETING

A neighborhood meeting was held October $2^{\text {nd }}, 2023$ for this requested zoning change due to the change of zoning from PUD-MU (Commercial and Industrial) to R3 (Residential) and MU-RC (Regional Commercial). The Town mailed notices to all property owners within 800 feet of the subject property. Any comments from the meeting or received by the staff will be shared with the Planning and Zoning Commission on the hearing date.

## STAFF ANALYSIS

This property is located along a main activity corridor for the Town, with expectations of medium to high intensity and development. Updates to existing roadway infrastructure are planned to accommodate this level of development along both Hwy 60 and High Plains Blvd, with right of way dedications and road improvements for this and surrounding properties. The zone change to MU-RC and R-3 allows for the Town and developers to work off of the updated 2023 Town of Johnstown Land Use Code. The MU-RC (Regional Commercial) zoning is in alignment with the high density/intensity areas of the Town of Johnstown Comprehensive Use Plan on the western and southern sides of the subject property. The R3 zoning to the north and east is in alignment with the medium density/intensity areas of the Town of Johnstown Comprehensive Use Plan and provides a transition from lower density residential areas to the north and east.

## RECOMMENDED PLANNING AND ZONING COMMISSION FINDINGS AND MOTIONS

It is recommended that Planning and Zoning Commission send a positive recommendation to Town Council that the requested zoning of MU-RC and R3 for Vista Commons be approved based upon the finding that the proposed zoning will allow appropriate levels of development to occur in alignment with the Town's Comprehensive Plan.

## Recommended Motion

Based on the application materials received and analysis and presentation at the hearing, the Planning \& Zoning Commission finds that the request for MU-RC and R3 zoning for Vista Commons furthers the Johnstown Area Comprehensive Plan, and is compatible with surrounding neighborhoods, and therefore moves to recommend to the Town Council approval of the request for MU-RC and R3 zoning for this area based upon the finding as stated above.

## Alternate Motions

Motion to Deny: "I move that the Commission recommend to the Town Council denial of the MU-RC and R3 zoning for Vista Commons based upon the following..."

## The Community That Cares

johnstown.colorado.gov




[^0]:    Mountain View 7:15 am 07/01/2024 2024 Opening Day - Background Timing Plan: AM Peak Hour

[^1]:    Mountain View 7:15 am 07/01/2024 2024 Opening Day - Background Timing Plan: AM Peak Hour

[^2]:    Mountain View 7:15 am 07/01/2024 2024 Opening Day - Background Timing Plan: AM Peak Hour

[^3]:    Mountain View 4:45 pm 07/01/2024 2024 Opening Day - Background Timing Plan: PM Peak Peak

[^4]:    Mountain View 4:45 pm 07/01/2024 2024 Opening Day - Background Timing Plan: PM Peak Peak

[^5]:    Mountain View 7:15 am 07/01/2024 2024 Opening Day - Total Timing Plan: AM Peak Hour

[^6]:    Mountain View 7:15 am 07/01/2024 2024 Opening Day - Total Timing Plan: AM Peak Hour

[^7]:    Mountain View 7:15 am 07/01/2024 2024 Opening Day - Total Timing Plan: AM Peak Hour

[^8]:    Mountain View 4:45 pm 07/01/2024 2024 Opening Day - Total Timing Plan: PM Peak Peak

[^9]:    Mountain View 4:45 pm 07/01/2024 2024 Opening Day - Total Timing Plan: PM Peak Peak

[^10]:    Mountain View 7:15 am 07/01/2045 2045 Future Year - Background Timing Plan: AM Peak Hour

[^11]:    Mountain View 7:15 am 07/01/2045 2045 Future Year - Background
    Timing Plan: AM Peak Hour

[^12]:    Intersection Summary

[^13]:    Mountain View 7：15 am 07／01／2045 2045 Future Year－Background Timing Plan：AM Peak Hour

[^14]:    Mountain View 7:15 am 07/01/2045 2045 Future Year - Background Timing Plan: AM Peak Hour

[^15]:    Mountain View 4:45 pm 07/01/2045 2045 Future Year - Background Timing Plan: PM Peak Hour

[^16]:    Mountain View 4:45 pm 07/01/2045 2045 Future Year - Background
    Timing Plan: PM Peak Hour

[^17]:    Mountain View 4:45 pm 07/01/2045 2045 Future Year - Background Timing Plan: PM Peak Hour

[^18]:    Mountain View 4:45 pm 07/01/2045 2045 Future Year - Background Timing Plan: PM Peak Hour

[^19]:    Mountain View 4:45 pm 07/01/2045 2045 Future Year - Background Timing Plan: PM Peak Hour

[^20]:    Mountain View 4:45 pm 07/01/2045 2045 Future Year - Background Timing Plan: PM Peak Hour

[^21]:    Mountain View 7:15 am 07/01/2045 2045 Future Year - Total
    Timing Plan: AM Peak Hour

[^22]:    Intersection Summary

[^23]:    Mountain View 4:45 pm 07/01/2045 2045 Future Year - Total
    Timing Plan: PM Peak Hour

[^24]:    Mountain View 4:45 pm 07/01/2045 2045 Future Year - Total
    Timing Plan: PM Peak Hour

[^25]:    Mountain View 4:45 pm 07/01/2045 2045 Future Year - Total
    Timing Plan: PM Peak Hour

[^26]:    Mountain View 4:45 pm 07/01/2045 2045 Future Year - Total Timing Plan: PM Peak Hour

[^27]:    ${ }^{1}$ HYDROZONE:
    VL -Very Low Water Use - indicates a plant that requires little to no additional water once established.
    L - Low Water Use - indicates a plant that requires an additional 3 gallons of water per square foot per season.
    M - Moderate Water Use - indicates a plant that requires an additional 10 gallons of water per square foot per season.
    H - High Water Use (None Selected) - indicates a plant that requires continual water.
    ${ }^{2}$ EXPOSURE: S-Shade PS-Part Shade FS-Full Sun

[^28]:    ${ }^{1}$ Revere at Johnstown, Filing No. 1 Transportation Impact Study, Delich Associates, September 2020.

[^29]:    Key: Stop-Controlled Intersection: Lev el of Service

[^30]:    ${ }^{2}$ Weld County Engineering and Construction Criteria, Atkins, January 2021.
    ${ }^{3}$ State Highway Access Code, State of Colorado, March 2002.

